



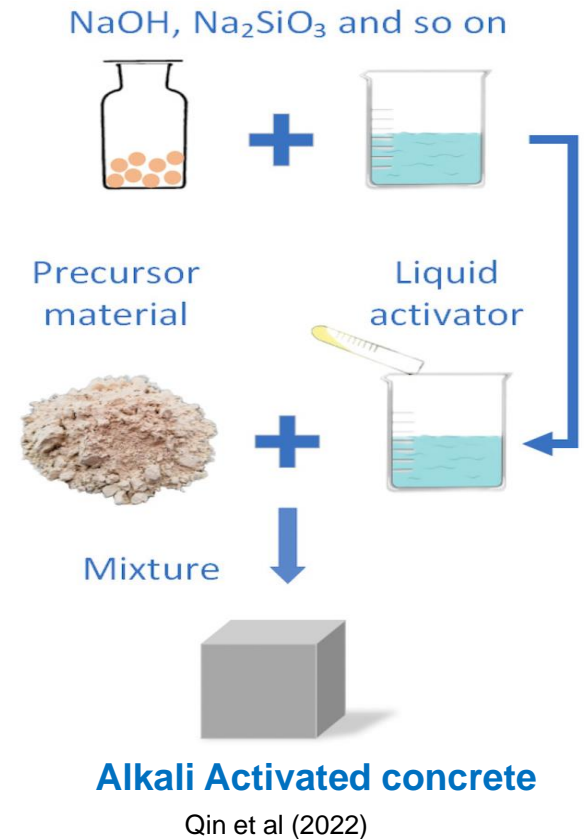
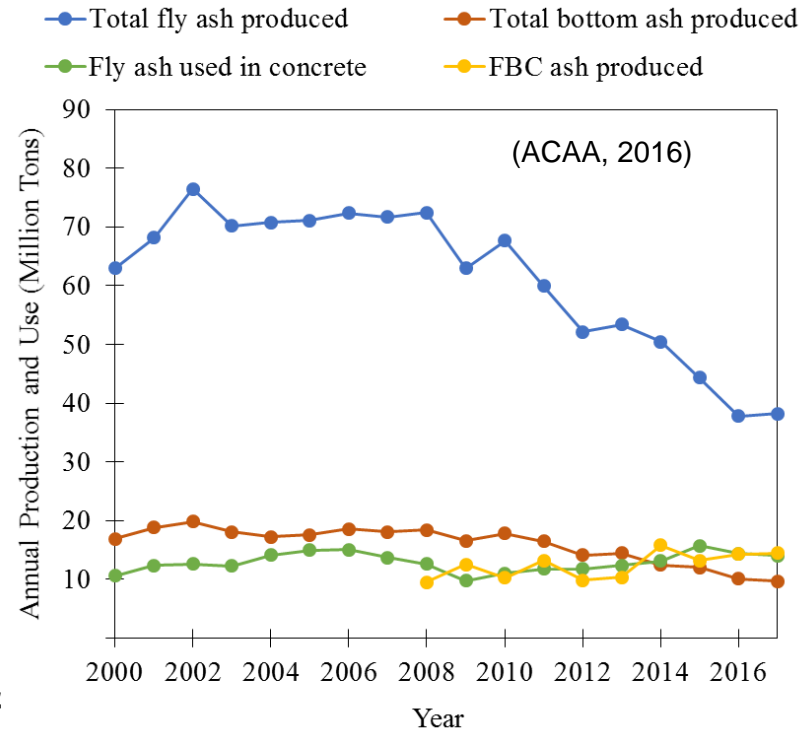
ASR Resistance of Ground Bottom Ash-Based Alkali-Activated Concrete and the Prospect of Using MCPT for the Evaluation

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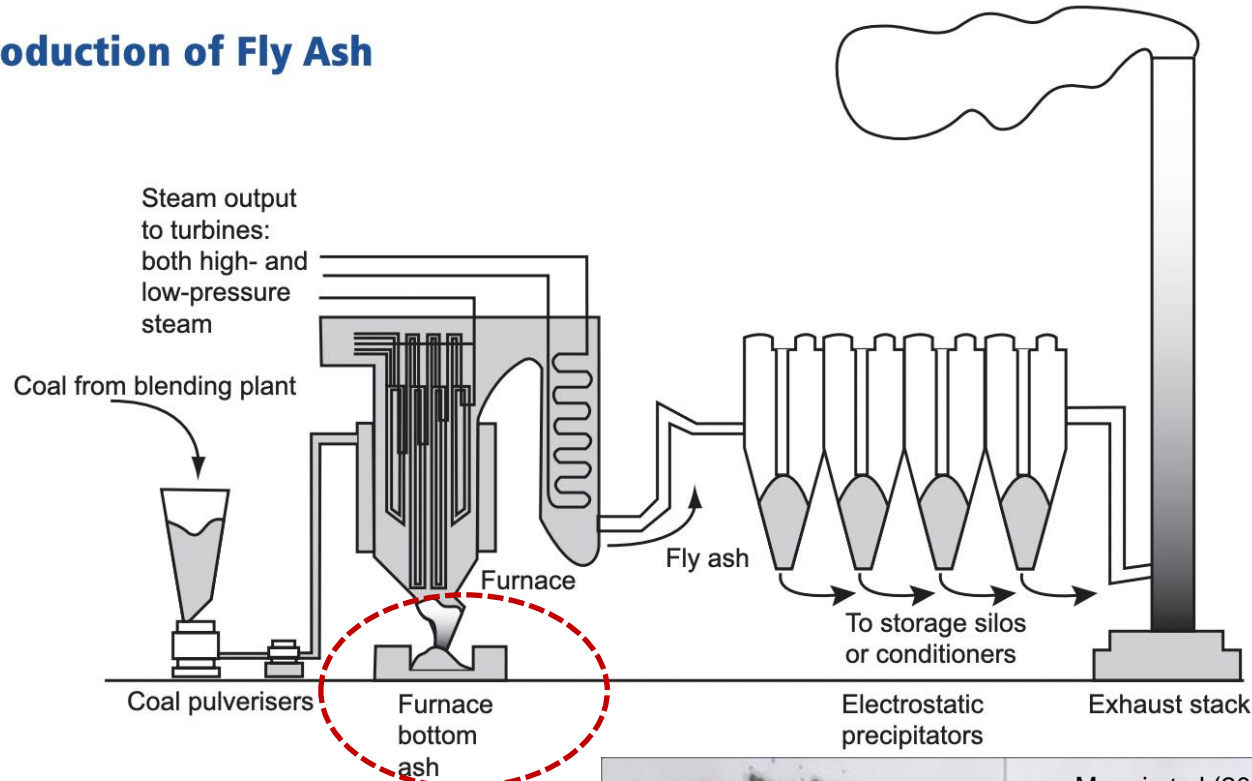
Background & Motivation

- By 2025, global cement demand - 4.7 billion metric tons (an increase of 2.9% per year).
- Eco-efficient binders – Alkali-activated materials
- Fly ash and GGBS – popular precursors
- Rapid decommissioning of thermal power plants in and competitive use of Slag as SCMs.
- Ground bottom ashes - emerging unconventional precursor for Alkali activated systems.



Ground Bottom Ash

Production of Fly Ash



- Similar chemistry to Fly Ash
- Potential SCM
- Coarser particle size than FA

Andrabi (2007)



Mangi et al (2019)



Background – ASR in AACs

- Concern on ASR Susceptibility in Alkali activated system

The **high alkalinity of pore solution**

- Deleterious ASR may be unlikely to occur as the **reaction products in alkali-activated concrete** typically contain no/limited **calcium hydroxide (CH)**
- Literature mainly reports the ASR resistance behavior of alkali-activated **fly ash** and **slag systems**.

- The realm of ASR-related performance for **unconventional precursors** is largely unexplored.

Objective
*ASR resistance in
GBAs-based AAC
using MCPT*



Materials - Binders



GBA1

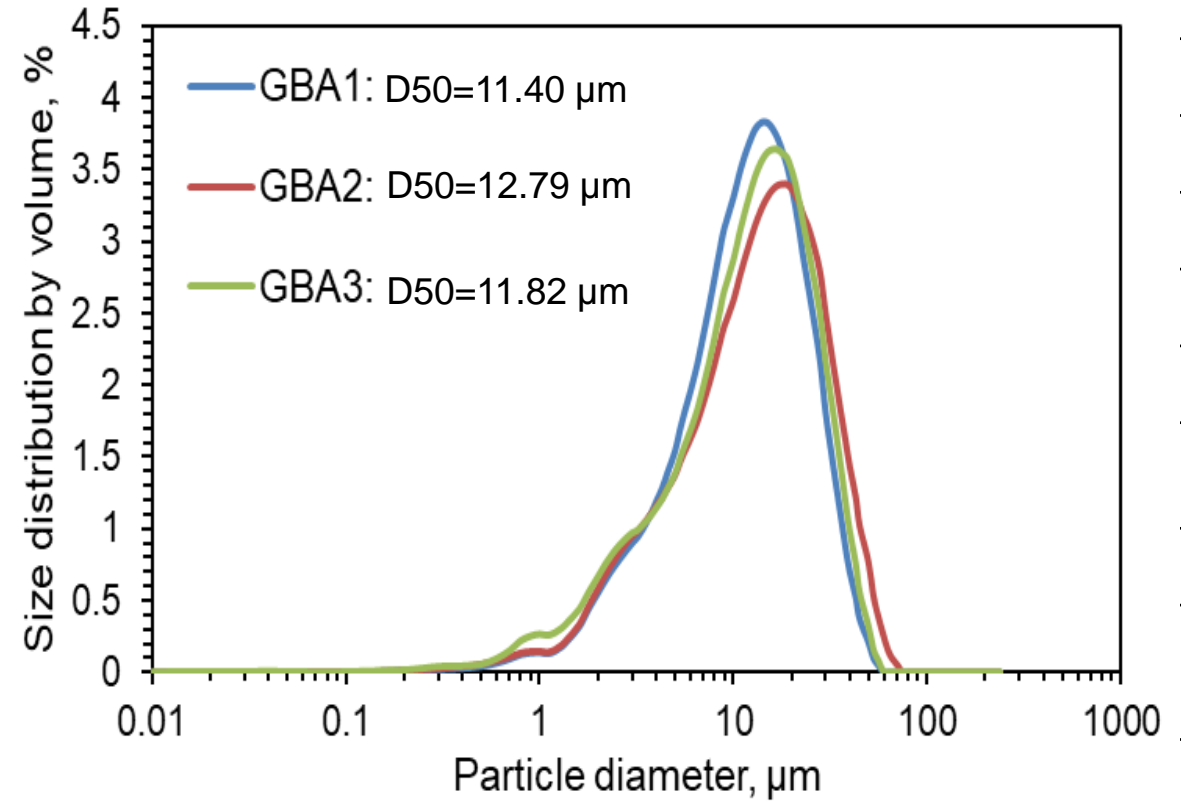


GBA2



GBA3

| Oxide (%) | GBA1 | GBA2 | GBA3 |
|--------------------------------|-------|-------|-------|
| SiO ₂ | 47.6 | 57 | 42.9 |
| Al ₂ O ₃ | 20.07 | 17.89 | 16.62 |
| Fe ₂ O ₃ | 24.12 | 5.84 | 7.46 |
| CaO | 2.82 | 11.5 | 23 |
| SO ₃ | 0.08 | 1.23 | 0.38 |
| Na ₂ Oeq | 2.01 | 0.97 | 1.29 |
| LOI (<spec. 6% ash to 10%NP) | 1.08 | 0.01 | 1.37 |



| | | | |
|------------------|--------------|--------------|--------------|
| Augite | - | - | 3.4% |
| Amorphous | 77.1% | 67.3% | 52.7% |

Materials

| Material | Binder Composition | Activator Solution | | |
|----------|--------------------|---|----------------|-------------------|
| | | Na ₂ O (% by mass of binder) | Silica Modulus | Solution / binder |
| AAC 1 | GBA1 + 3% CH | 9.25 | 1.25 | 0.6 |
| AAC2 | GBA2 + 3% CH | 9.25 | 1.25 | 0.6 |
| AAC3 | GBA3 + 3% CH | 9.25 | 1.25 | 0.6 |

3 types of reactive aggregates are considered (classified using C1778 on basis of C1260 results)

- a) Roaring Spring sand – R1 (moderately reactive)
- b) Spratt limestone – R2 (high reactive)
- c) Jobe Sand – R3 (very high reactive)

Normal river sand (R0) and limestone CA : to establish control specimens

Experimental - MCPT

- AASHTO T 380 -19
- Less effort in **processing aggregates** required
- Shorter / rapid test with a duration of **56 days** (majority of aggregates).
- Used for evaluating **Aggregate reactivity** and **efficiency of SCMs** in mitigating ASR.
- Subjected to **:1 N NaOH solution and 60°C**
- Non-reactive sand is used for reactive CA and vice versa
- 2"x2"x11" specimens

| Expansion at 56 days ($\mu\epsilon$) | Efficiency of mitigation |
|--|---------------------------------|
| <200 | Effective |
| 200 -250 | Uncertain |
| >250 | Ineffective |

Scope Matrix

| Mixes | s/b | Binder (kg/m ³) | Activator (kg/m ³) | Fine aggregate (kg/m ³) | Coarse aggregate (kg/m ³) | Water (kg/m ³) |
|-------|-----|-----------------------------|--------------------------------|-------------------------------------|---------------------------------------|----------------------------|
| AAC1 | 0.6 | 480 | 288 | 824 | 1052 | 16 |
| AAC2 | 0.6 | | 288 | 852 | 1088 | 16 |
| AAC3 | 0.6 | | 288 | 813 | 1035 | 16 |

Concrete mix proportions:

35% paste volume

Fine aggregate (FA) /

Total aggregate (TA) =

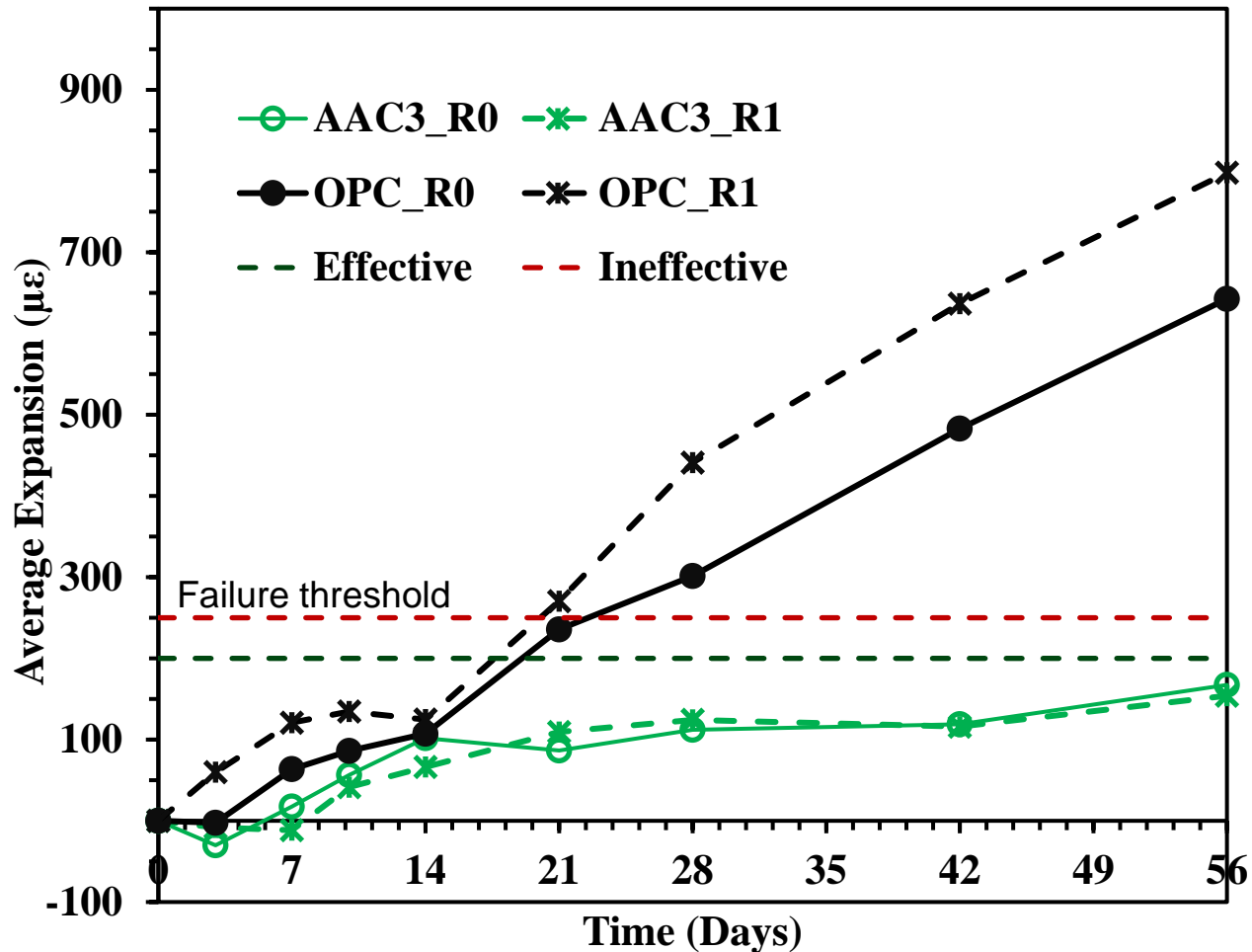
0.45 by volume

Compressive strength > 40 MPA

- OPC control specimens - MCPT recommendations- w/c=0.45 in combination with all tested aggregate categories.

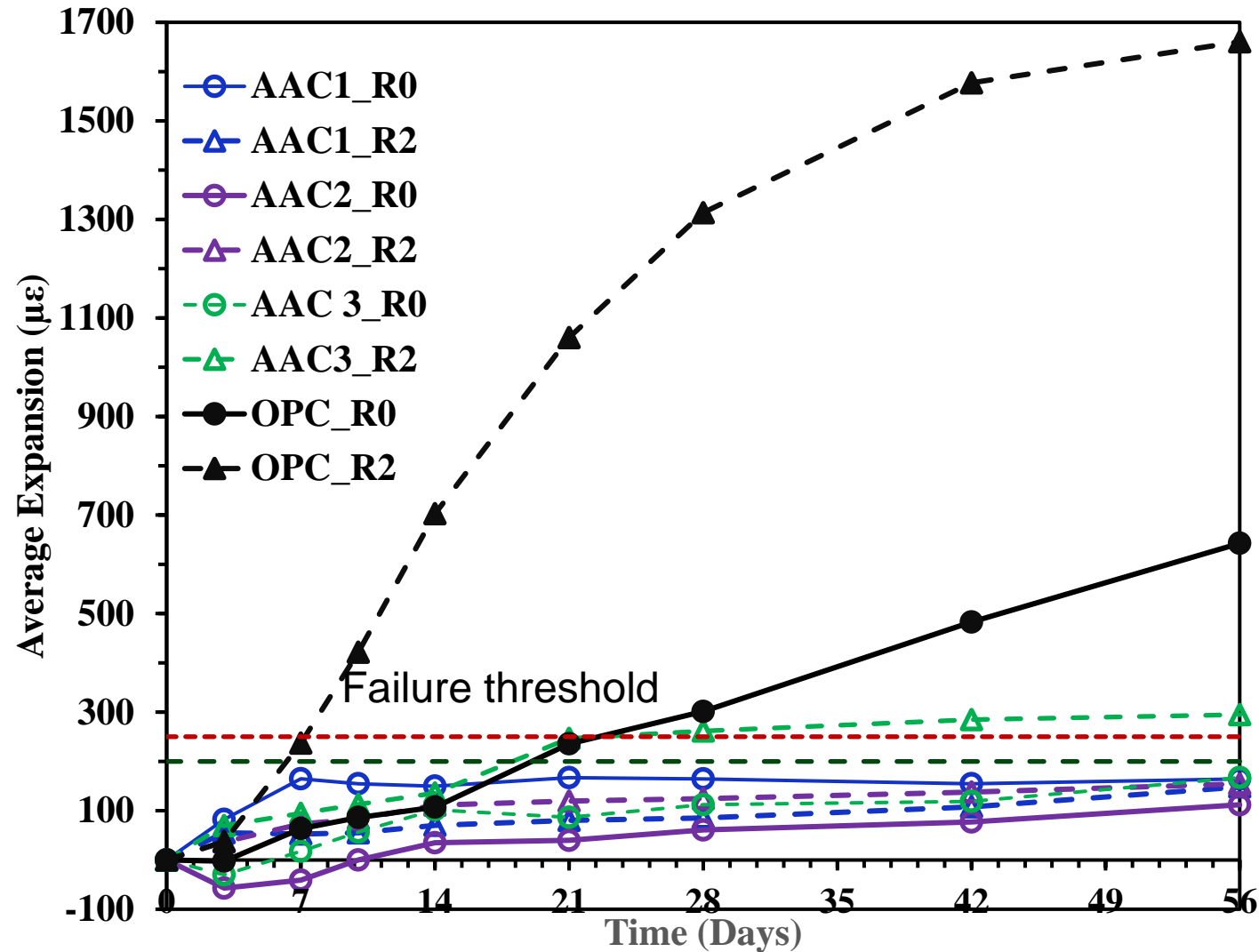
| Mixes | R0 | R1 | R2 | R3 |
|-------|----|----|----|----|
| AAC1 | ✓ | - | ✓ | - |
| AAC2 | ✓ | - | ✓ | - |
| AAC3 | ✓ | ✓ | ✓ | ✓ |
| OPC | ✓ | ✓ | ✓ | ✓ |

Results: Moderately reactive aggregates



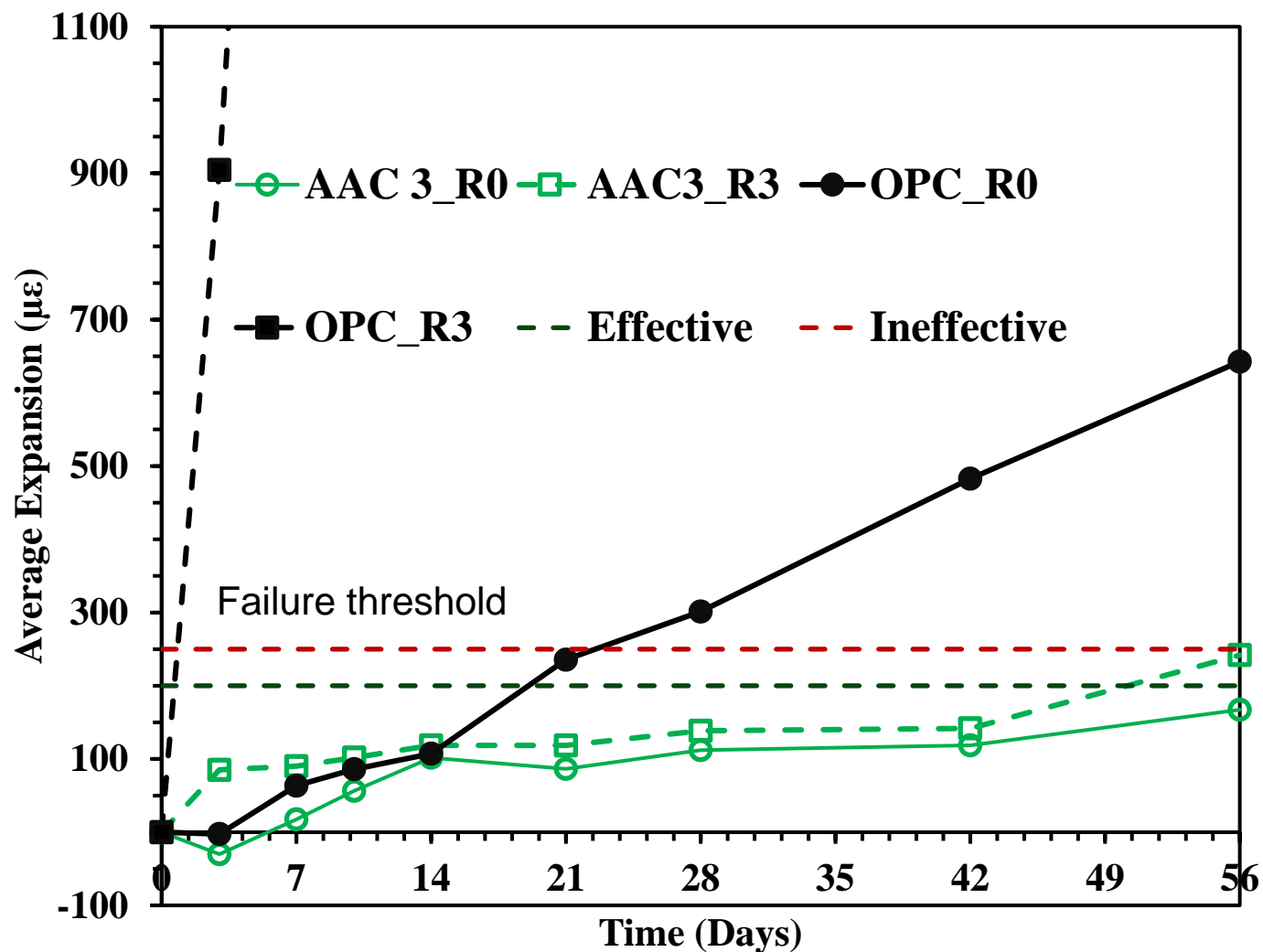
- AAC specimens demonstrate **superior resistance**.
- AACs, perform similarly in the presence and absence of moderately reactive aggregates.

Results: Highly reactive aggregate



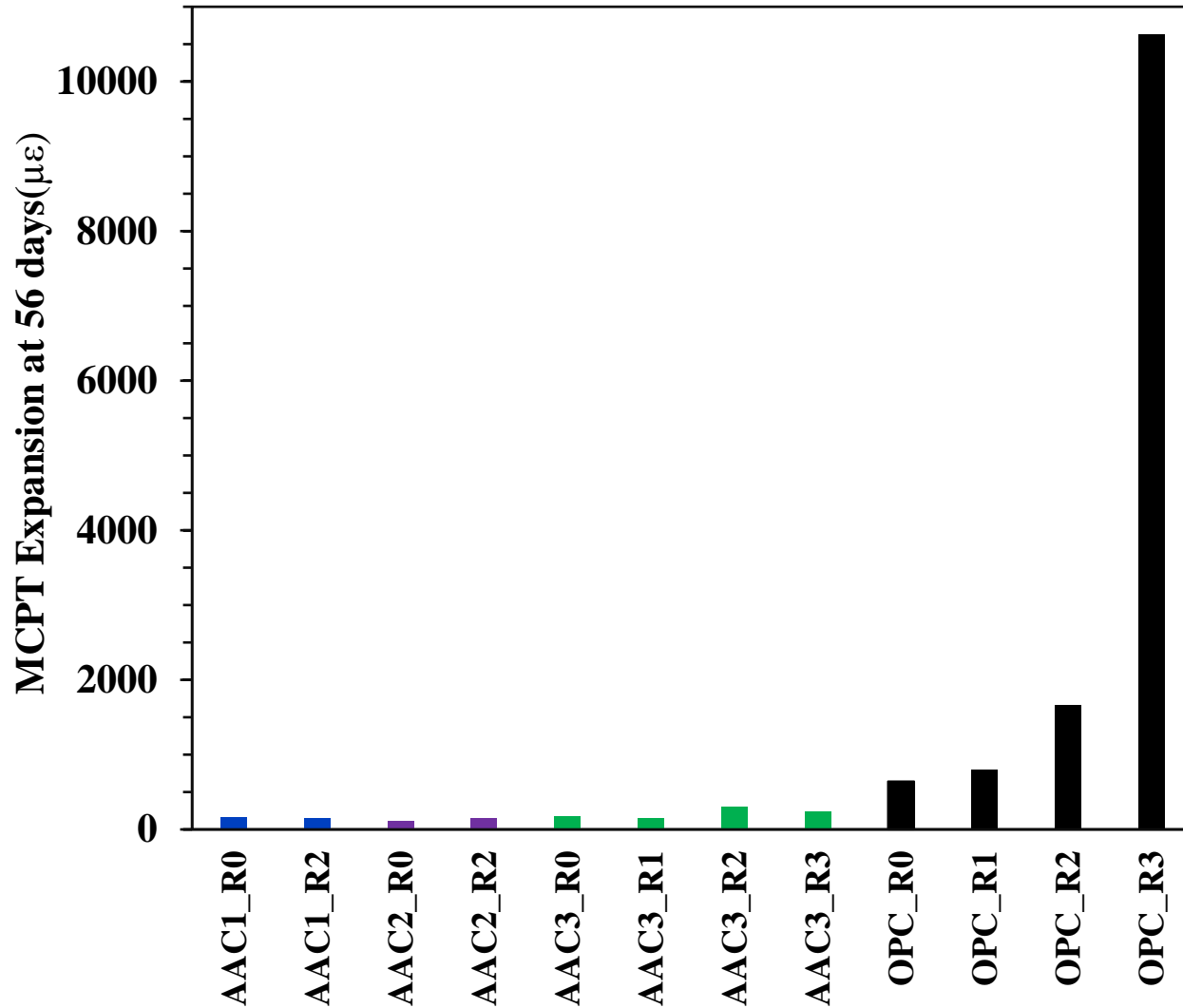
- AAC specimens : significantly low levels of expansion
- Potentially due to alkali binding, low availability of calcium and improved microstructure

Results: Very high reactive Sand



- OPC with R3: very high expansion in order of 10,000 με.
- Nothing alarming for AAC despite high solution/ binder ratio.

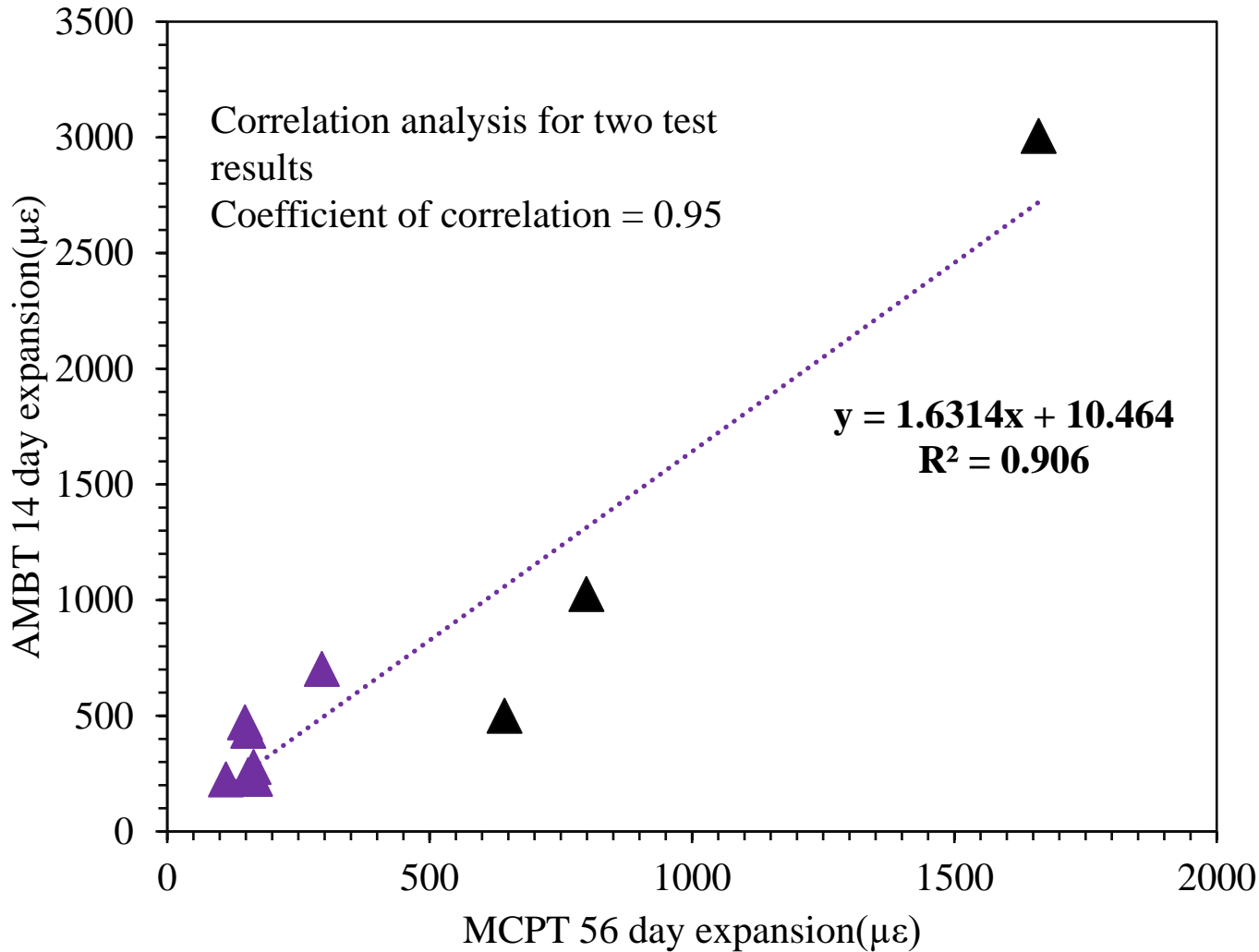
Comparison of performance



$$\text{ASR Susceptibility Index} = \frac{\text{OPC Expansion at 56 days}}{\text{AAC Expansion at 56 days}}$$

| For moderately reactive (R1) type aggregate | |
|--|----|
| AAC3 | 5 |
| For highly reactive (R2) type aggregate | |
| AAC1 | 11 |
| AAC2 | 11 |
| AAC3 | 6 |
| For very highly reactive (R3) type aggregate | |
| AAC3 | 44 |

MCPT vs. AMBT



- The results obtained from 56 days of MCPT demonstrate a strong correlation with the 14-day expansions of AMBT.
- Both test methods indicate that AACs exhibit a high level of resistance to expansion induced by ASR

Summary and Future Works

- All the tested AACs demonstrated superior resistance to ASR.
- Potentially due to **alkali binding**, **low levels of calcium**, and **refined pore structure**
- Conventional concrete **expanded** evidently in presence of reactive aggregates due to expansive ASR gel.
- AACs can be potentially used as an **ASR mitigation strategy** in locations constrained to reactive aggregates.
- **MCPT** even though with **a conservative threshold** apparently suits the context of predicting ASR-related deterioration for GBA-based AACs.
- No **visible signs of distress** were noted for AACs.

Future Works

- Correlating **the pore solution chemistry** of AACs to ASR resistance.
- Relation of **surface and bulk electrical resistivity** with the ASR behavior of AACs.

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