Long Term Monitoring of Bridge Decks in Montana

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The Problem

- Closely spaced transverse cracks on numerous bridges in western Montana
- In some cases, through deck penetrations developed
- Young bridge decks < 8 years</p>
- Mostly in the alpine regions





The Problem





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The Investigation

- Construction Document and Literature Review
- Field Inspections
- Instrumentation
- Laboratory Testing
- Finite Element Modeling (FEM)

Field Inspections

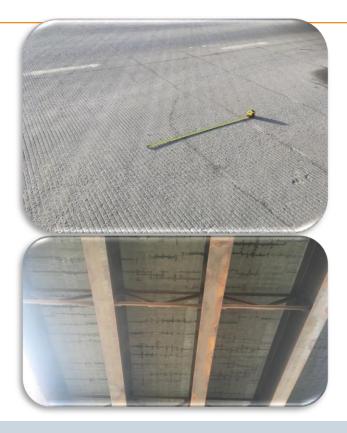
- 14 bridge decks inspected
 - Crack mapping density/severity
 - Delamination survey
 - Ground penetrating radar
 - Infrared thermography
 - Drone survey
 - Concrete core extraction
 - Bridge deck documentation

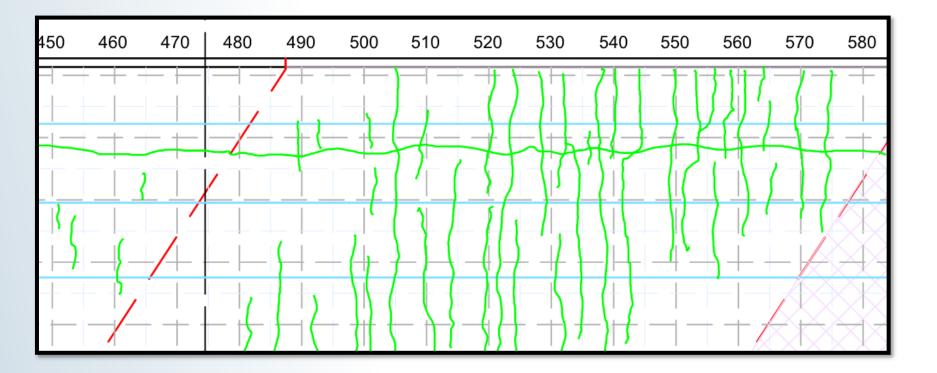


Field Inspections

- Overall Visual Rating = 3
- Transverse cracks at 3 to 4 feet
- Crack width 20 to 30 mils

Crack density	Crack Severity
(ft/ft2)	(mil*ft/ft2)
0.19	3.87
0.14	2.45
0.17	3.15

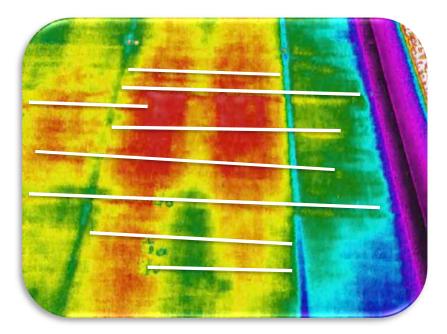






Field Inspections – IR/Drones

- Drone survey with infrared thermography (IR)
- IR could pick up cracks as tight as 5 mils
- Cracking density but not cracking severity



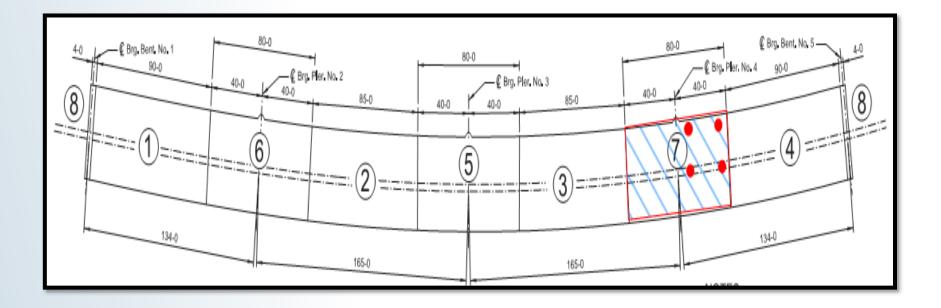
Field Inspections - Summary

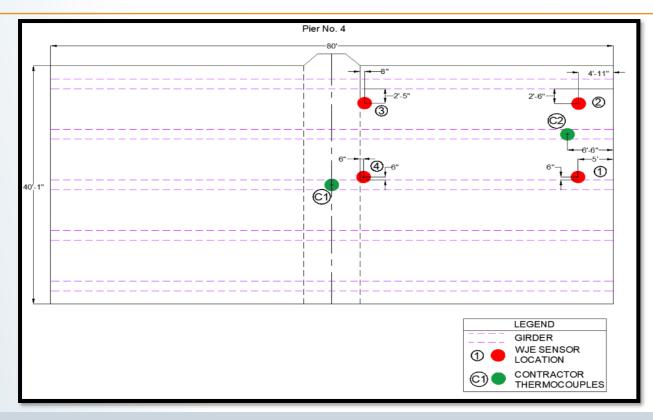
- No delamination found
- Transverse cracks align with reinforcing steel
- Most transverse cracks are full thickness
- The following factors did not yield any consistent trends in the development of transverse cracking severity: bridge bearing type, span length, span bearing type, placement location, and placement length.
- Very early age and late age development of transverse cracks

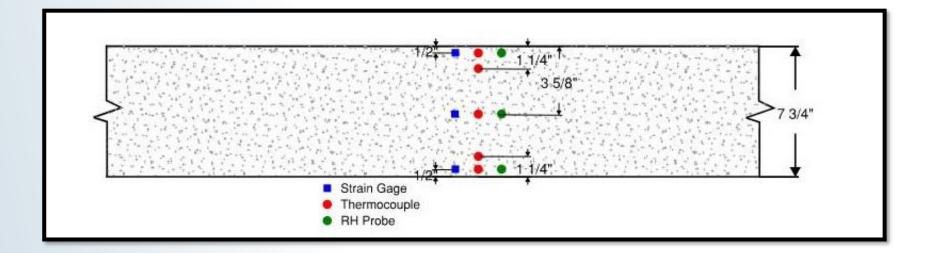
- Assess the impact of environmental changes on the internal deck temperatures, relative humidity (RH), and strains.
- Instrumentation:
 - Strain (vibrating wire SG's)
 - Temperature
 - Relative Humidity (resistive)
 - Ambient conditions



- temperature, relative humidity, wind speed, and solar radiation





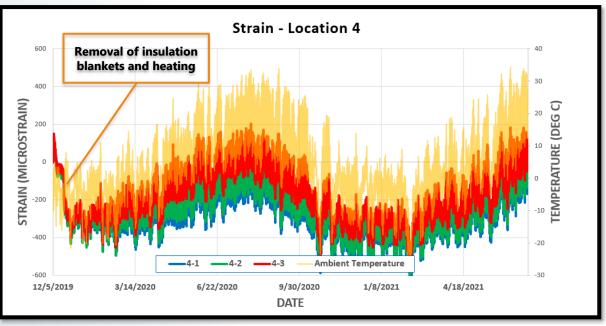


Vibrating wire strain gages RH Sensors Thermocouples

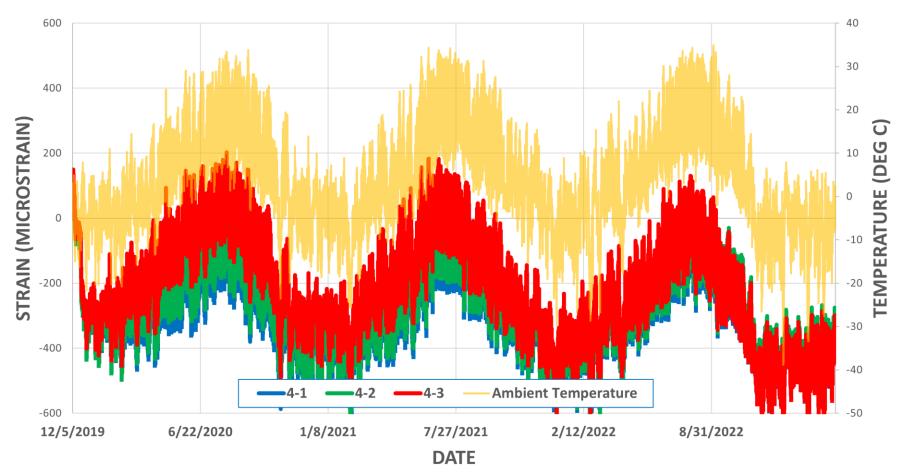


Instrumentation - Strain

Compressive strain developed in deck after removal of insulation and heating

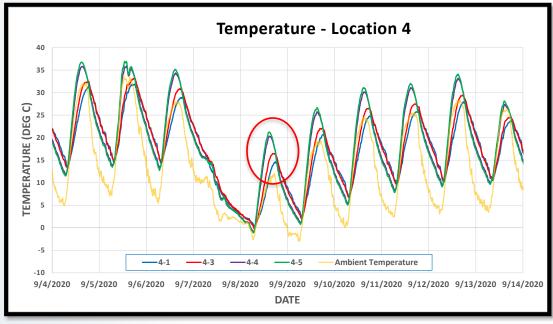


Strain - Location 4



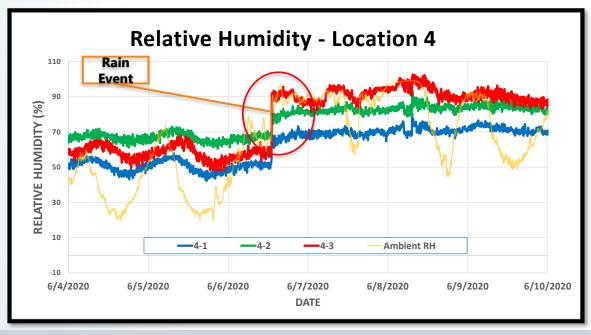
Instrumentation - Temperature

- Large daily temperature changes observed ($\Delta T = 55$ to 60°F) Temperature gradient within the deck, 20F



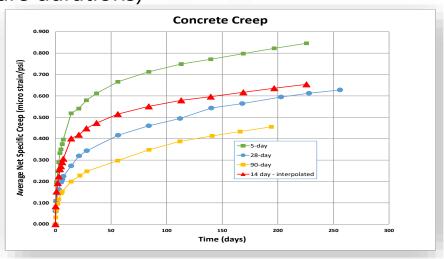
Instrumentation - Relative Humidity

 Large daily RH changes observed, increase from 20 to 100 % within 24 hours - RH gradient within the deck up to 30% RH



Laboratory Testing

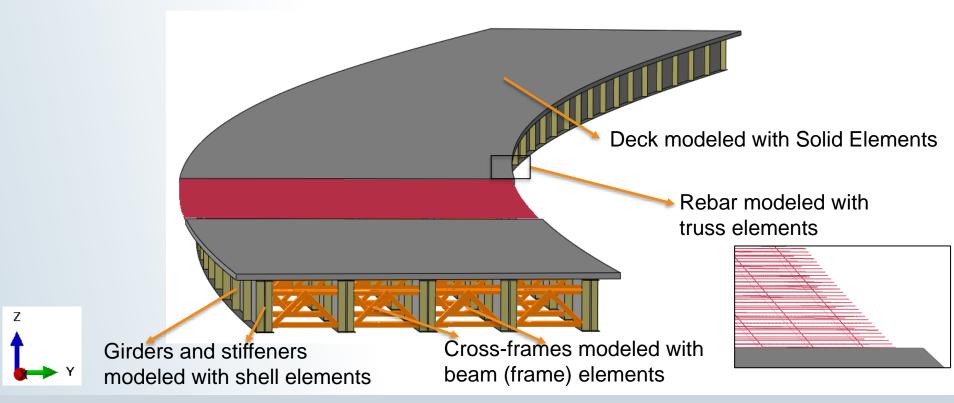
- Mix designs were tested in the laboratory for the following:
 - Compressive, splitting, MoE, and flexural strength versus maturity
 - Drying shrinkage (varying moist cure durations)
 - Creep (various loading ages)
 - Coefficient of thermal expansion



Finite Element Modeling (FEM)

- Full scale, 3D Model of bridge deck created in Abaqus/CAE 2020
- Same bridge as instrumentation
- 4-span bridge, 5 steel plate girders with 7.75" composite deck
- FE model included full-length deck geometry, girders, and lateral braces

Finite Element Modeling (FEM)

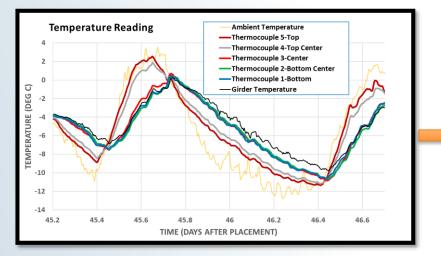


Finite Element Modeling (FEM) - Scenarios

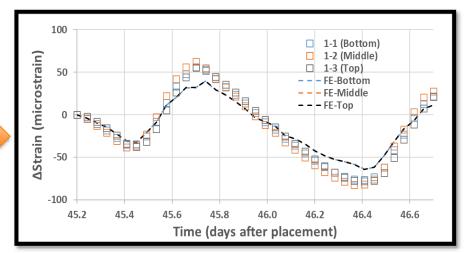
- Later-age Analyses (typically >90 days after placement)
- Early-age Analyses (4 hours to 14 days after placement)
- Factors investigated:
 - Drying shrinkage
 - Temperature histories (sharp drop or increase)
 - Relative humidity (moisture) histories (sharp drop or sharp increase)
 - Length of wet-curing time with summer and winter placements
 - Sensitivity on deck thickness and girder restraint

Finite Element Modeling (FEM) - Validation

- Model was validated against field strain gauge data
 - The goal was to verify the global FE model trend VS. actual field behavior



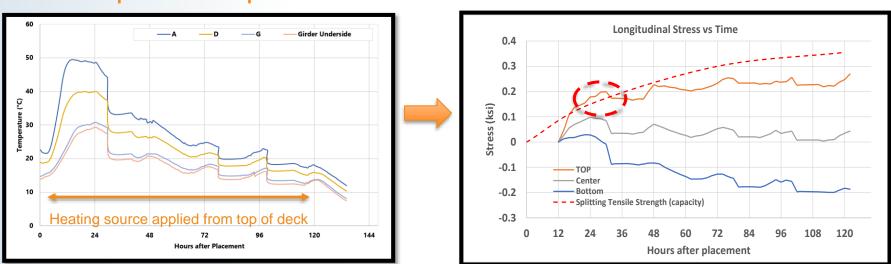
Temperature input for FE Model



Strain output from FE Model against field-measured strain data

FEM - Early-age Winter Placement

Heating from top of the deck



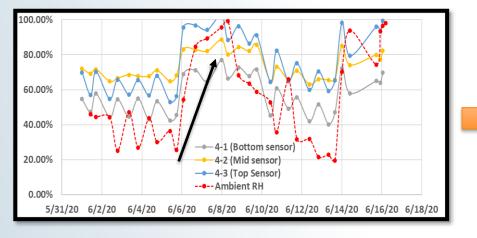
Winter placement temperature

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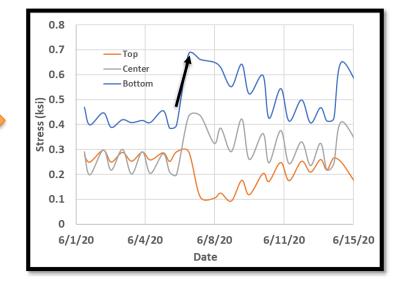
Average Resultant Longitudinal Stress vs time

FEM - Later-age Moisture Gradient

Recorded Relative Humidity from the Field



Stress vs time



- Higher RH gradient existed after rain event.
- Increase in tensile stresses at the bottom of the deck ~300 psi

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Finite Element Modeling (FEM) - Summary

- Drying shrinkage can contribute up to 300 psi in tensile stress
- Large temperature rises can create an increase in tensile stress by as much as 400 psi, underside of deck
- Large changes in relative humidity can create an increase in tensile stress by as much as 300 psi, underside of deck
- Winter curing, heating from below is preferred to heating from the top

- Three primary goals of recommendations:
 - 1. Reduction in drying shrinkage
 - 2. Reduction in thermal gradients
 - 3. Reduction in moisture gradients

Reduction in volumetric movement

Mixture Proportions

- w/cm of 0.42 to 0.45
- Limit total cementitious to 600 lb./yd³ or less
- Use of slag cement and fly ash
- Lower plastic concrete temperatures to < 75F
- Optimized aggregate gradation, likely needed for reduction in cementitious content
- Larger top sized aggregate

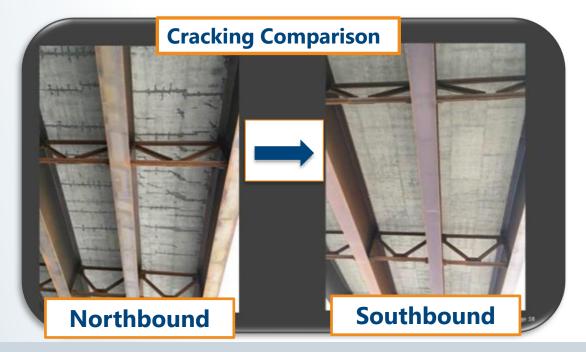
Design and Construction Practices

- Summer curing:
 - Installation of insulated blankets after peak hydration
 - Removal of insulation blankets after 4 to 5 days
- Winter curing:
 - Heat cure from underside is preferable and provides additional pre-compression benefit
- Reduction in moisture gradient installation of thin-polymer overlays at later ages ~ 6 to 9 months after construction

Design and Construction Practices

- Increase design thickness of decks to 8 inches minimum
- Modify specifications to require staggering of top and bottom transverse reinforcing mats

Cracking comparison, before and after WJE's recommendations



Thank you!

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