

Shrinkage Cracking Control of Concrete Using Non-Metallic Rebars and Meshes

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Plastic Shrinkage Cracking

- Has been studied over the last 60 years
- One of the earliest flaws in concrete
- Caused by restrained volumetric changes in the first 3-8 hours after casting
- Reduces concrete durability and structural service life

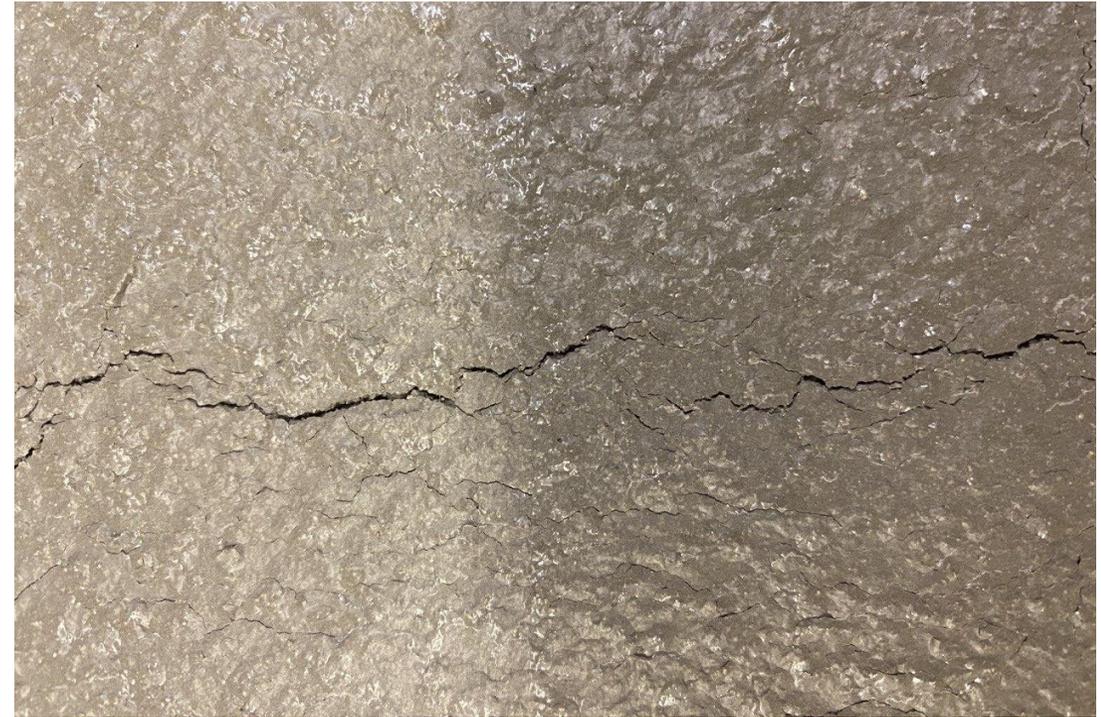


Fig. 1. Plastic shrinkage crack

Driving causes of plastic shrinkage

- Settlement of solid particles
- Bleeding
- Evaporation
- Capillary action
- Surface finishing

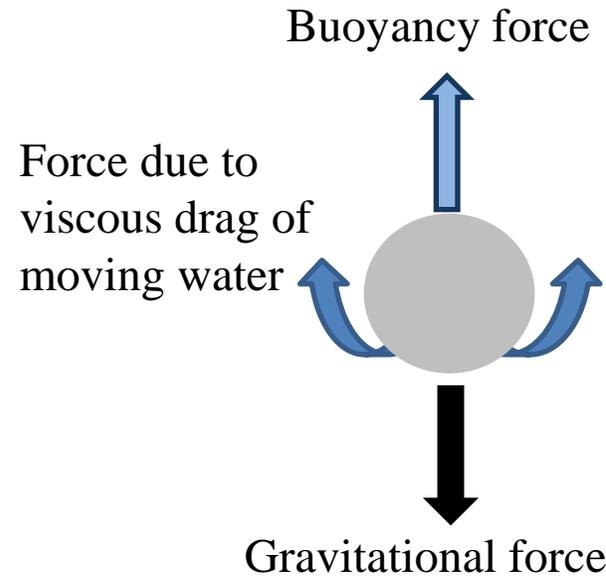


Fig. 2. Major forces acting on a solid particle in fresh concrete

But how to control it?

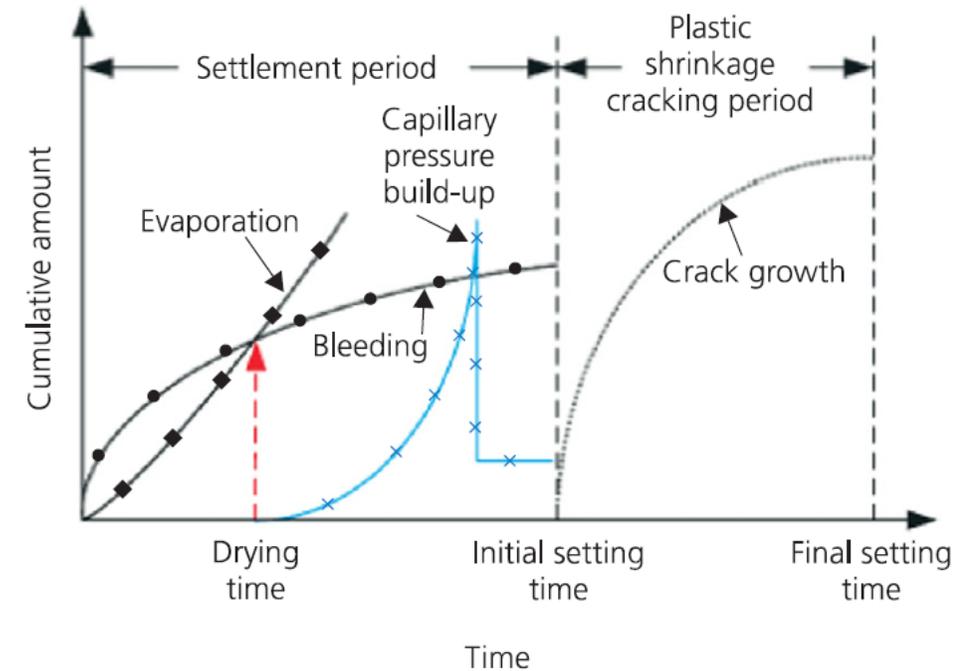


Fig. 3. Sequence of events prior to and after cracking

Shrinkage cracking control

- Alternative concrete mixture
- Concrete surface sealers
- Curing techniques
- Fiber Reinforced Polymer (FRP) reinforcement

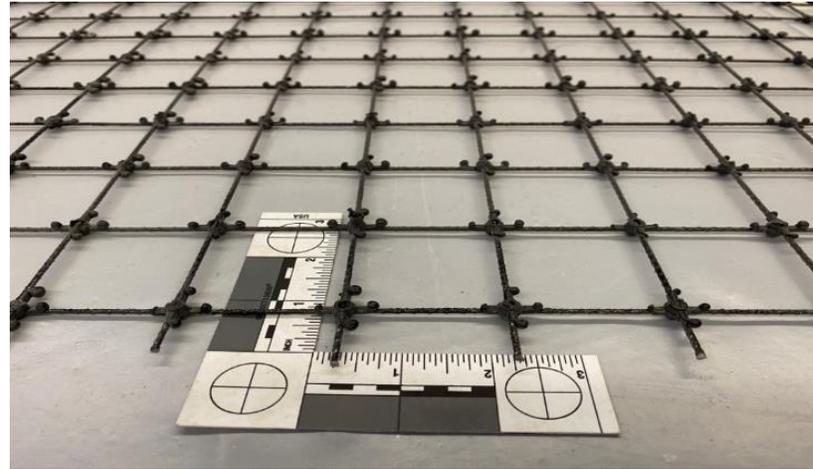


Fig. 4. FRP Mesh



Fig. 5. FRP bars

Why FRP?

What is a sustainable material?

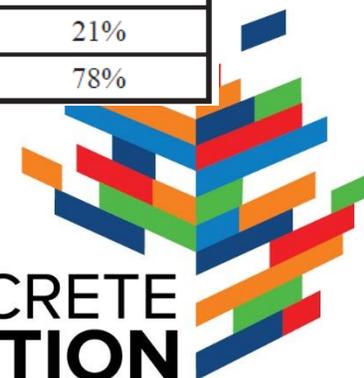
- Last longer
- Require lower energy
- Emit less carbon dioxide

Environmental impact of FRP

- Increase service life of structure
- Decrease CO₂ emissions
- Lower energy consumption

Reference	Type of FRP	FRP Replacement % to Steel Rebars	Environmental Impact Category	Reduction % in the Environmental impact due to using FRP rebars
Garg and Shrivastava, (2019)	GFRP	100%	Global Warming (CO ₂ emissions)	43%
Garg and Shrivastava, (2019)	GFRP	100%	Energy Consumption	47%
Garg and Shrivastava, (2019)	BFRP	100%	Global Warming (CO ₂ emissions)	40%
Garg and Shrivastava, (2019)	BFRP	100%	Energy Consumption	50%
Garg and Shrivastava, (2019)	CFRP	100%	Global Warming (CO ₂ emissions)	39%
Garg and Shrivastava, (2019)	CFRP	100%	Energy Consumption	32%
Van Loon et al. (2019)	E-glass	46%	Shadow Cost	28%
Van Loon et al. (2019)	Flax	46%	Shadow Cost	39%
Van Loon et al. (2019)	Kenaf	39%	Shadow Cost	36%
Inman et al. (2017)	BFRP	100%	Global Warming (CO ₂ emissions)	62%
Inman et al. (2017)	BFRP	100%	Ozone Depletion	21%
Inman et al. (2017)	BFRP	100%	Human Toxicity	78%

Table 1. Comparison of LCA results of replacement of the conventional steel reinforcement by FRP composites



Plastic Shrinkage Cracking Test

Modified ASTM C1579

- Based on guidelines of ASTM C1579 and AC521
- Evaluate FRP as secondary reinforcement
- To compare of FRP and steel for controlling shrinkage cracks

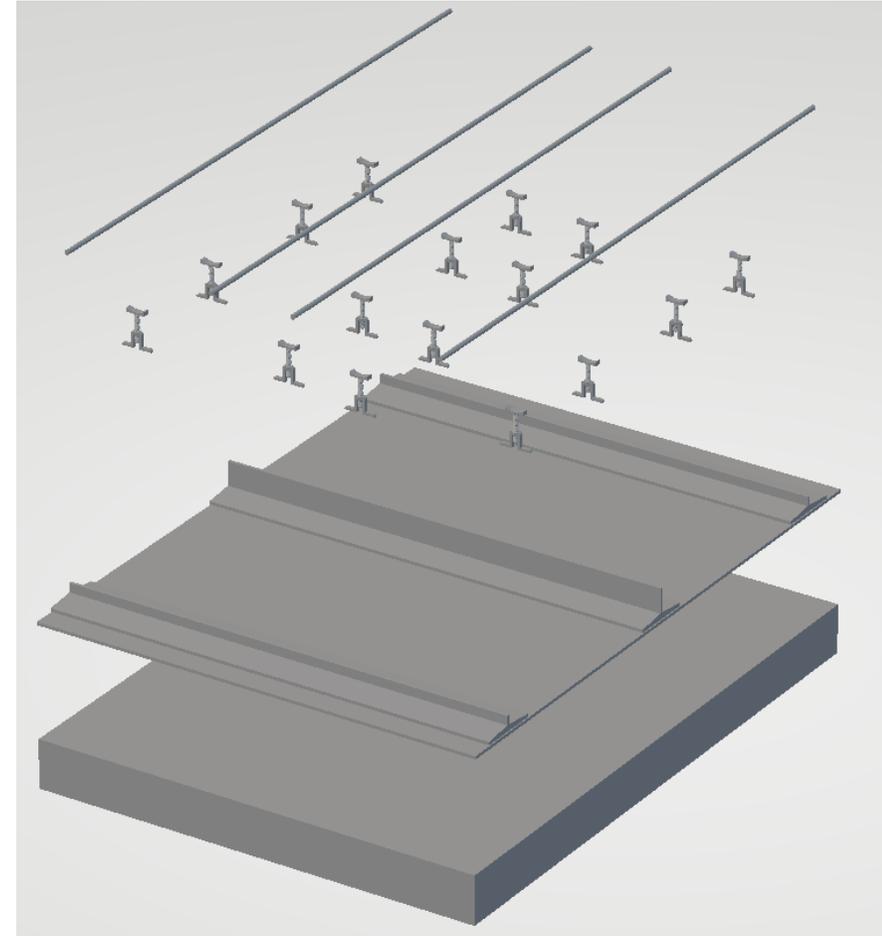


Fig. 6. Specimen, riser and restraints, reinforcement, and chairs



Plastic Shrinkage Cracking Test

Mold

Each specimen contains 2 sections

1. FRP
2. Steel

Wind tunnel

- To obtain a laminar wind flow
- Accelerate evaporation and provide a severe condition



Fig. 7. Mold and reinforcement configuration

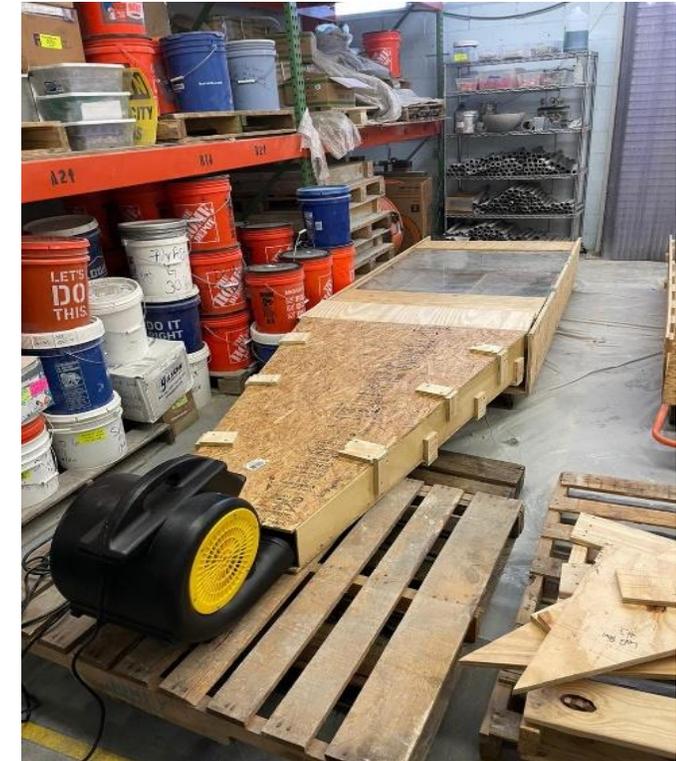


Fig. 8. Wind Tunnel Design

Plastic Shrinkage Cracking Test

Influential parameters

- Environmental variables (wind speed, RH, ambient temperature)
- Material characteristics (reinforcement and concrete)
- Restraint conditions (at both ends)
- Stress concentration (riser)
- Clear concrete cover (0.75 in.)
- Base roughness (steel plate)
- Workmanship

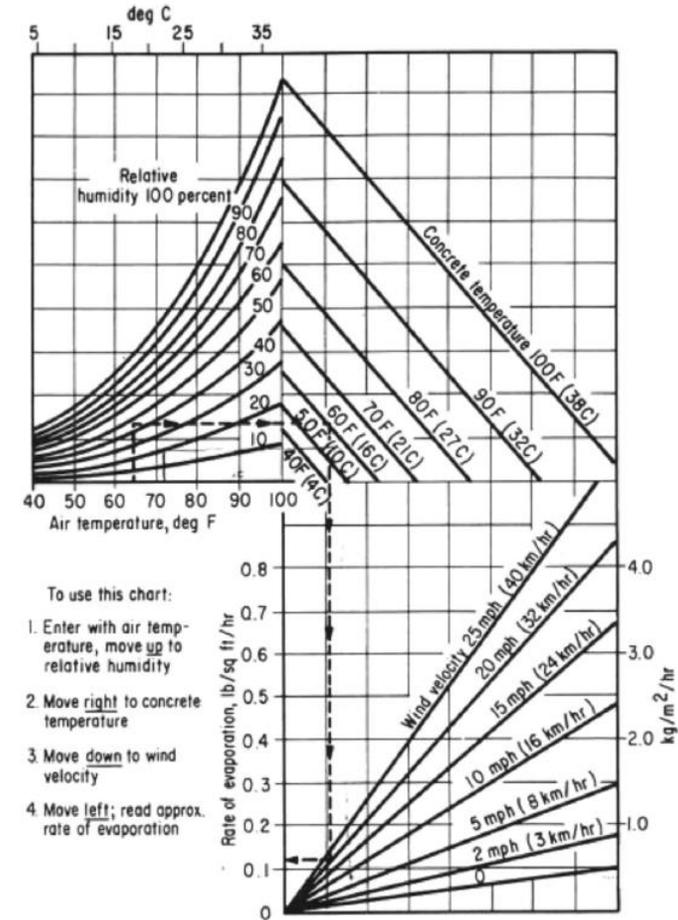


Fig. 9. Nomograph for estimating rate of evaporation



Environmental variables

- Wind speed measured at midspan using an anemometer
- RH and ambient temperature measured using a thermometer in the wind tunnel

Measurement	Temperature (inside the wind tunnel) [°F]	RH (inside the tunnel) [%]	Wind Speed (at Mid span) [m/s]
1	72.2	66.0	4.4
2	71.6	61.0	4.6
3	72.2	62.0	4.4
4	72.9	59.0	4.6
5	72.9	59.0	4.5
6	72.4	60.0	4.6
7	73.3	62.0	4.7
8	73.6	59.0	4.6
9	73.3	60.0	4.5
10	72.9	59.5	4.5
11	72.5	60.0	4.5
12	72.9	62.0	4.8
13	72.9	60.0	4.6
14	72.9	60.0	4.5
15	72.9	60.0	4.5
16	72	60.0	4.5
Average	72.7	60.6	4.5

Table 2. Environmental variables measurement

Material characteristics

- FRP
 1. Tensile properties (ASTM D7957 and D7205)
 2. Surface enhancement (surface roughness)
 3. Bond strength (ASTM D7957 and D7913)
- Concrete
 1. Compressive and tensile strength
 2. Time of setting (ASTM C403)
 3. Concrete temperature (implementing sensors in concrete)
 4. Rate of evaporation

Specimen ID		Tensile Force P_{Max}		Nominal Area A_{nom}		Ultimate Strength $F_{tu_{nom}}$		Modulus of Elasticity E_{nom}		Ultimate Strain ϵ_{tnom}
		KN	Kips	mm^2	in^2	Mpa	Ksi	GPa	Msi	%
A-35L1 TNS 01		72.06	16.20	71	0.11	1015.4	147.3	53.4	7.75	1.9
A-35L1 TNS 02		71.42	16.10			1006.4	146	54.2	7.86	1.86
A-35L1 TNS 03		74.30	16.70			1046.9	151.8	51.1	7.41	2.05
A-35L1 TNS 04		73.72	16.60			1038.8	150.7	52.4	7.61	1.98
A-35L1 TNS 05		72.06	16.20			1015.4	147.3	53.3	7.73	1.91
	Average	72.71	16.36			1024.6	148.6	52.9	7.67	1.94
		1.23	0.27			17.32	2.49	1.18	0.17	0.08
	CV%	1.69	1.65			1.69	1.67	2.24	2.23	3.87
	Guaranteed Tensile Load	69.02	15.55							

Table. 3. FRP bar tensile properties



Evaluate Cracking Control

Crack Measurements

- Depth
- Width
- Length
- Area

Crack reduction ratio is computed to evaluate FRP performance:

$$CRR = \left[1 - \frac{\text{Average Crack Width of section reinforced with FRP}}{\text{Average Crack Width of section reinforced with steel}} \right]$$

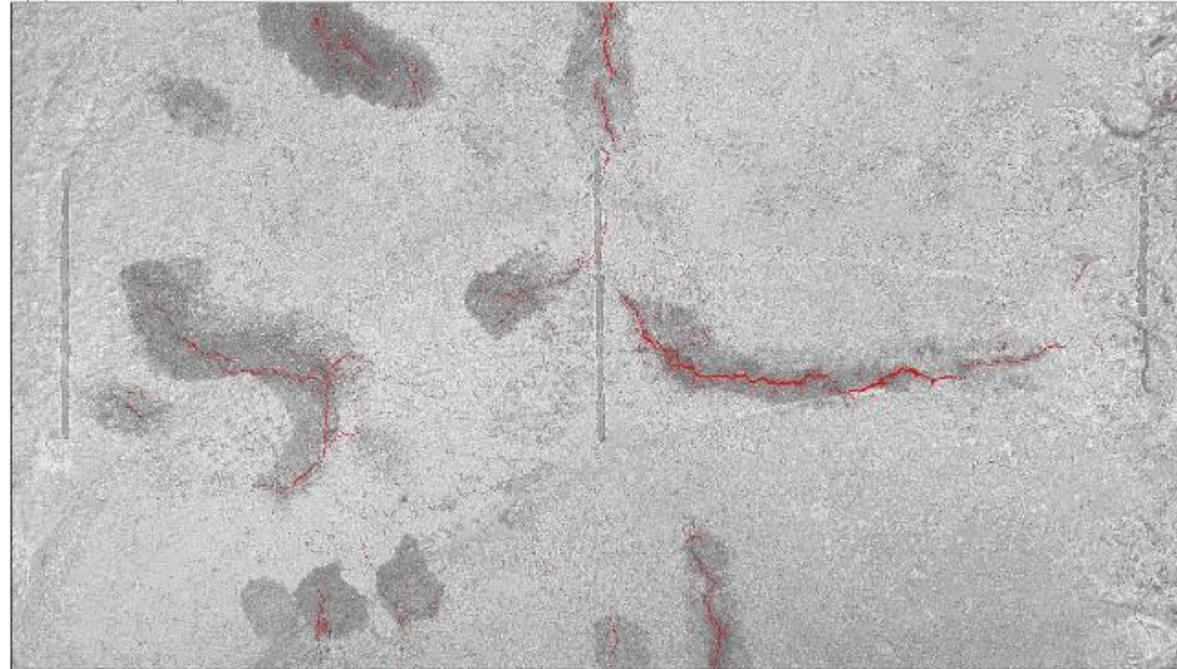


Fig. 10. Processed image of concrete surface



Fig. 11. Concrete core

Conclusions (expected)

- 1- Provide experimental evidence to show efficiency of FRP to control plastic shrinkage cracks
- 2- Establish feasible equivalency between FRP and steel
- 3- Minimum FRP reinforcement as secondary reinforcement (temperature and shrinkage reinforcement)
- 4- A standard test method to evaluate FRP as secondary reinforcement

Thank



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