Flexural Performance of Dually Reinforced 3D Concrete Printed Beams

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RESEARCH FOCUS:

Structural performance of modular 3D concrete printed components under moderate seismic action



Selecting Suitable Reinforcement Strategies

Cumulative number of papers relating to respective reinforcement strategies



Selected Strategies

1. Entrained fibres

- Most publicised strategy
- High level of compatibility
- Ease of application

2. Permanent formwork

- 3rd most publicised strategy
- Most often implemented in practice
- Readily available





Dual Reinforcement Strategy

6mm entrained HM-PP microfibres







FRPC mixture constituent proportions

Constituent	kg
PPC Suretech CEM II/A-L 52.5 N	562
Durapozz Fly Ash (Class F)	162
Micro Silica Fume	81.4
Fine Aggregate (Malmesbury)	1144
Water	256
Superplasticizer	0.6% by binder mass
VMA	0.3% by binder mass
6 mm HM-PP Microfibres	1% by mixture volume

HM-PP microfibre properties

Description	Value
Young's Modulus (E _f)	30 GPa
Yield Stress (f _t)	1200 MPa
Diameter (d)	15 µm
Length (L)	6 mm

Y10 in-laid conventional reinforcement



Loading Configurations

Configuration 1





Summary of dimensions used

Dim.	Value	Units	Dim.	Value	Units
а	280	mm	е	502	mm
b	338	mm	h	260	mm
с	25	mm	w	260	mm
d	230	mm	L	840	mm

Configuration 2



Member & LVDT Frame





Experimental Results





Configuration 3







- Ultimate bending capacity = 45.3 kN.
- Maximum mean mid-span deflection = **7.4 mm**.
- Strain-softening post-peak response displayed.
- Symmetric single vertical bending crack.
- **3.6 x** Amplified load-carrying capacity.

1.

- No noticeable bond-slip or pull out between rebar & printed matrix.
 - 103 kN: Diagonal shear crack, thus shear dominant failure.
- 2. **164 kN**: Limited deformability of printed matrix \rightarrow Strain localisation \rightarrow rebar yields.
- **3.** Multiple smaller flexural cracks: Increased deformation + strain redistribution.
- 4. Interlayer delamination: Excessive deformation & curvature.
- 1. 93 kN: Diagonal shear crack, thus shear dominant failure.
- 2. 124 kN: Vertical bending crack \rightarrow rebar yields \rightarrow plateau.
- 3. Interlayer delamination: Excessive deformation & curvature.
- 4. Smaller flexural cracks: Increased deformation + strain redistribution.
- 5. Brittle ult. failure: shear-flexure crack: lack of shear links.
- 6. Horizontal crack: debonding due to cracking, large curvature & reduced contact area on location of rebar placement.

Numerical Simulation Frameworks

Expected Failure Mechanisms in 3DCP Elements



Continuum Model











2D Plane Stress Simplification



Calculating the Effective Plane Stress Thickness

$$t = \frac{A_{\sec A-A}}{h} = \frac{42727}{260} = 164 mm$$
$$\Delta_{CF} = \frac{L_{w,eff}}{L_{w}} = \frac{26}{30} = 0.86$$
$$t_{eff} = \Delta_{CF} \cdot t = 141 mm$$

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Numerical Simulation Parameters: Continuum Model

Model Description:

To exploit the **anisotropic Rankine-Hill continuum model** available in the **DIANA FEA** package, two elastic parameters (E, v), seven strength parameters (f_{tx} , f_{ty} , f_{cx} , f_{cy} , α , β , and γ), and five inelastic parameters (G_{fx} , G_{fy} , $G_{fc,x}$, $G_{fc,y}$, and κ_p) are required.

Summary of Parameters						
Isotropic Elastic Parameters						
E	21.9	GPa	V	0.2	-	
Orthotropic Strength Parameters						
x-direction				y-direction		
f _{tx}	2.45	N/mm ²	f_{ty}	1.25	N/mm ²	
f _{cx}	45.1	N/mm ²	f _{cy}	38.2	N/mm ²	
Unitless Strength Parameters						
α	0.35	β	-1	Y	0.525	
Orthotropic Inelastic Parameters						
Crack bandwidth User S		Specified				
G _{fx}	0.956	N/mm	G _{fy}	0.063	N/mm	
G _{fc,x}	27.07	N/mm	$G_{fc,y}$	26.17	N/mm	
Kn	0.002	mm/mm				

van den Heever et al., Mechanical Characterisation for Numerical Simulation of Extrusion-based 3D Concrete Printing, Journal of Building Engineering, in Review, 2021a.

Shear stress contribution to tensile failure

$$\alpha = \frac{f_{tx} \cdot f_{ty}}{\tau_u^2}$$

 τ_u is the interface shear-slip (Mode 2) capacity equal to 2.96 MPa Shear stress contribution to compressive failure

$$\gamma = \frac{f_{cx} \cdot f_{cy}}{\tau_u^2}$$

 τ_u is the material pure shear (Mode 2) strength equal to $1.5f_{c,y}$ MPa Coupling of normal stress values for compressive failure

 $\beta = -1$

Default assumed in the absence of experimental data

Numerical Simulation Parameters: Discrete Interface-based Model

Model Description:

The combined-cracking-shearing-crushing (CCSC) interface model available in DIANA FEA is implemented to define the IRs in the 3DCP composite. The intralayer filaments are prescribed an isotropic hypo-elastic total strain-based rotating crack (TSC) constitutive relation, also available in the DIANA FEA package.

Interface Parameters (CCSC)			Continuum Parameters (TSC)		
Parameter	Value	Units	Parameter	Value	Units
f _{t,j}	1.25	N/mm ²	E	21900	N/mm ²
G ^I _{f,i}	0.063	N/mm	v	0.2	-
C _c [°]	2.96	N/mm ²	ρ	2.15E-06	T/mm ³
Friction angle (φ)	36.87	degree	Crack orientation	Rotating	
Dilatancy angle (ψ)	0	degree	Tensile curve	Exponential	
Res. friction angle (ϕ_r)	36.87	degree	f _t	2.45	N/mm ²
Conf. normal stress	-1	N/mm ²	G ^I f	0.956	N/mm
Exp. Deg. Coeff.	1	-	Crack bandwidth	User specified	
G ^{II} _{fc}	0.296	N/mm	Poisson ratio reduction	Damage based	
G _{cc}	26.17	N/mm	f _c	45.1	N/mm ²
k _{nc}	1.00E+06	N/mm ³	G _{cc}	27.07	N/mm
k _{sc}	4.17E+05	N/mm ³	Reduction due to lateral cracking		
C _{ss,j}	3.5	-			NO
f _{c,j}	38.2	N/mm ²	Confinement increase		No
<u>Кр</u>	0.01	N/mm ²			

Summary of Parameters

van den Heever et al., Numerical Modelling Strategies for Reinforced 3D Concrete Printed Elements, Additive Manufacturing, in Review, 2021b.

Numerical Simulation Results: Continuum Model



Numerical Simulation Results: Discrete Interface-based Model



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Conclusions

- Both entrained fibre & in-laid reinforcement methods are compatible with 3DCP
- In-laid reinforcement provides 3.6 x amplified load-carrying capacity (p_s = 0.43%) attributed to strain-hardening, displays sufficient bond and anchorage, permitting strain redistribution and providing ductility.
- Strain-softening in observed for the singularly reinforced beam while strain-hardening for dually reinforced member
- Pure bending of singularly reinforced members is accurately simulated by both numerical simulation strategies (peak load capacity < 4% & excellent agreement shown in cracking patterns and softening regimes).
- Pure bending of dually reinforced members is accurately simulated by both numerical simulation strategies (peak load capacity < 9% & respectable agreement shown in cracking patterns and post-peak regimes).
- Reasonable agreement is observed under eTPB (exhibiting a maximal load capacity <= 14%).
- From the numerical results it is evident that the shear stress is over estimated and additional experimental calibration is required.
- The similarities in both the force-displacement response and crack patterns attained across all simulations indicate that the respective FE simulation strategies apply to 3DCP



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