The Challenges and Opportunities of New Materials and Advanced Numerical Tools for the Development of Innovative and Sustainable Built Environment

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SUMMARY

- 1 Motivation
- 2 Materials of complementary functionality for the development of innovative construction systems
 - 2.1 LEGOUSE
 - 2.2 PONTALUMIS
 - 2.3 PRESLABTEC

3 – Modelling and design

- 3.1 Main models for the simulation and design of FRC structures
- 3.2 Blind Simulation Competition (BSC)
- 3.3 Design applications
- 3.4 Design challenges and opportunities

4 - Conclusions



1 – MOTIVATION

- Explore the potentialities of materials of complementary properties and functionalities for developing more sustainable constructions systems, such is the case of fiber reinforced cement-based materials (FRC) and fiber reinforced polymers (FRP);
- How reliable are existing advanced numerical approaches for assisting on the optimum use of FRC and FRP regarding innovation and sustainability on the built environment – the role of Blind Simulation Competitions.



2 – MATERIALS OF COMPLEMENTARY FUNCTIONALITY FOR THE DEVELOPMENT OF INNOVATIVE CONSTRUCTION SYSTEMS



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2.1 - LEGOUSE

Simply perforated plates:





• • • • • • • • • • • • • • **Progressive failure** 128 112 96 80 **(H**⁶⁴ 48 48 32 16 0 10 Slip (mm)

Pouring the second SFRSCC layer

















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Construction of sandwich panels prototypes for testing programs





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Testing real scale sandwich panels prototypes





















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Pseudo-static in-plane cyclic tests





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Construction of real scale prototype



CONVENTION



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Construction of real scale prototype







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Construction of real scale prototype







Real scale prototype of 100 m² area, capable of hosting 6 persons, built in the Portuguese city of Rio Maior.

Two weeks are required to build completely this modular construction, with a final cost per m² of 400€.



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2.2 - PONTALUMIS



2.2 - PONTALUMIS

Construction of the real scale prototype

Mechanical properties of the SFRSCC (avg. \pm std. dev.).

$E_{c,28}$	f_{cm}	f _{ct,L}	f _{eq,2} [MPa]	<i>f_{eq,3}</i>	$f_{R,1}$	$f_{R,2}$	<i>f_{R,3}</i>	<i>f_{R,4}</i>
[OI a]								
37.75±1.31	75.95±10.03	6.21±1.25	10.42+2.42	10.56±2.40	10.17±2.16	10.27±2.34	9.71±2.26	9.01±2.15



(a) manufacturing the GFRP profiles; (b) Transportation of the GFRP component; (c) casting the SFRSCC deck; (d) applying the stainless steel anchors and the epoxy layer; (e,f) placing the GFRP grid; (g,h) placing the footbridge on the final position



2.2 - PONTALUMIS

Tests on the real scale prototype

Uniformly distributed load along the entire span, centerd with the deck in a width of 1.20m a) **b**)

Uniformly distributed load in the central part of the span, in a length of about 2.7m, across the entire width of the deck

Uniformly distributed load in the central part of the span, in a length of about 5.1m, across the entire width of the deck

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8.8 kN/m², **76% higher** than the characteristic load preconized in Eurocode 1 for footbridges (5 kN/m²)



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2.2 - PONTALUMIS

Tests on the real scale prototype



 $q = 1.49 \text{ kN/m}^2$ (30% of the characteristic live load for footbridges defined in Eurocode 1)



The Findley's power law predicted appropriately (R²=0.963) the time deflection evolution: $\Delta(t) = \Delta_0 + m \cdot t^n$ with *m*=0.945 and *n*=0.197



2.2 - PONTALUMIS

Tests on the real scale prototype



	Mada	Frequency	Damping ratio					
Mode		Mean [Hz]	CoV [%]	Mean [%]	CoV [%]			
	1	6.40	0.28	1.89	18.69			
	2	8.16	0.01	1.26	11.77			
	3	12.13	0.63	1.96	16.28			
	4	20.78	12.28	1.57	62.08			
	5	22.16	6.14	0.92	20.59			
	6	23.74	0.09	0.76	11.65			
	7	31.11	0.28	1.74	13.29			
	8	42.37	0.10	0.70	11.21			
	9	45.96	0.06	0.53	11.62			
	10	51.69	0.14	0.47	14.21			
	11	60.21	0.04	0.79	24.45			
	12	68.29	0.05	0.43	14.99			
	13	69.24	0.25	0.17	26.56			
	14	77.18	0.06	0.57	19.01			
1	15	80.62	0.30	1.00	24.57			
	16	88 61	0.12	0 47	1945			



Pedestrian response tests: Very low probability of pedestrian discomfort due to the structural vibrations

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Modal identification test:

results of the first 16 mode shapes with

ambient excitation.





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2.2 - PONTALUMIS

Pedestrian bridge





The developed pedestrian bridge is in service since 2015 over the River Cáster in the Portuguese city of Ovar (S. Silvestre bridge)

The results of this project confirmed the potential of the proposed hybrid GFRP-SFRSCC structural system for footbridge applications.









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New slab system for the construction of building pavements



Demonstrating the structural performance by experimentally testing prototypes of real scale (bending tests)





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Technical data of the tested prototype

- Dead weight: 210kg/m²;
- Content of FRC: **0.08m³/m²**;
- Content of steel in the form of cold formed steel profiles: 18.21 kg/m²;
- At the maximum load, when the test was interrupted, the prototype was supporting an equivalent load of 44kN/m² with a
 ductile flexural failure mode.
- Linear phase, followed by an elasto-cracked stage of relatively high amplitude and small loss of stiffness up to the yield initiation of the longitudinal steel profiles, with subsequent hardening phase of very ductil behaviour.



Tests with prototypes of real scale



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PreSlabTec_calc

File Tools Help

2.3 - PRESLABTEC

PreSlabTec_calc

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ieome	try and materials	Loads Results	s (all slabs) Resu	ults							
	Height [mm]	ts [mm]	Strength class	Toughness class	Maximum instantaneous deflection (SLS) [mm]	Check deflection (SLS)	Check concrete compressive stress (SLS)	Check steel tensile stress (SLS)	Check bending (ULS)	Check shear (ULS)	Deta result
	220	1.5	C40/50	4c	9.95					Ń	v res
	220	1.5	C40/50	5b	8.95				X		v res
	220	1.5	C40/50	6a	8.83				X		v res
	220	2	C40/50	4c	8.78						v res
	220	2	C40/50	5b	8.15				X		v res
	220	2	C40/50	6a	8.07					Ń	v res
	220	3	C40/50	4c	7.48						v res
	220	3	C40/50	5b	7.19						v res
	220	3	C40/50	6a	7.16						v res
	260	1.5	C40/50	4c	5.52				X	<u>/</u> N	v res
	260	1.5	C40/50	5b	5.15					Ń	v res
	260	1.5	C40/50	6a	5.11				X	Ń	v res
	260	2	C40/50	4c	5.07					Ń	v res
	260	2	C40/50	5b	4.86				X		v res
	260	2	C40/50	6a	4.84					Ń	v res
	260	3	C40/50	4c	4.55						v res
	260	3	C40/50	5b	4.47						v res
	260	3	C40/50	6a	4.46						v res
	300	1.5	C40/50	4c	3.45				X		v res
	300	1.5	C40/50	5b	3.36				X	<u>/</u> !\	v res
	300	1.5	C40/50	6a	3.35						v res
•	300	2	C40/50	4c	3.28						v res
	300	2	C40/50	5b	3.25						v res
	300	2	C40/50	6a	3.25						v res
	300	3	C40/50	4c	3.06						v res
	300	3	C40/50	5b	3.05						v res
	300	3	C40/50	6a	3.05						v res

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2.3 - PRESLABTEC

Pilot unit







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CONCRETE CONVENTION A prototype of 12m span length is being tested according to the Eurocode 1 load conditions for long term deflection assessment.











3 – MODELLING AND DESIGN

3.1 – Main models for the simulation and design of FRC structures

- Smeared Crack Models (SCM)
- Discrete Crack Models (DCM)
- Concrete Damage Plasticity (CDP)
- Lattice Discrete Particle Models (LDPM)
- Others: based on moment-rotation, partition-of-unity, rigid-body-spring, and three-phase approaches

For design, the most used are SCM and CDP.



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at peak load have attained 40%, 113% and 600%, respectively

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Line load

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3.2 – 2nd BLIND SIMULATION COMPETITION (BSC)





Deadlines:

- 1) Information about the material properties will be provided up to 15/11/2021.
- 2) Participant must submit their report up to 23h:59 of 31/12/2021 (Spain time).
- 3) Experiments will be conducted on the 10 and 14 January 2022.
- 4) Conclusions about processing of results will be communicated by the end of February 2022.



Line load



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This design is being made by **FEMIX** software, written in C and C++ code

- Includes a large library of types of finite elements (FE);
- Set of point, line and surface (non-)linear springs to model diverse contact conditions with the supports;
- Several types of interface FEs to model inter-element contact;
- Embedded line FE to model reinforcement bars;
- > **Special embedded FE** for simulating fibers in FRCC context;
- Static and dynamic analysis, with material linear or nonlinear behavior;

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This design is being made by **FEMIX** software, written in C and C++ code

- In the same analysis several nonlinear models may be simultaneously considered;
- Time dependent phenomena like concrete maturation including creep and shrinkage;
- Newton-Raphson method, arc-length techniques and path dependent or independent algorithms;
- Direct and preconditioned conjugate gradient methods;
- Pre- and Post-processing by using GiD.





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Integrating hybrid reinforced concrete technology and advanced fEM-based numerical modelling for crack control in long concrete foundations without joints.





FRC design; testing and quality control; monitoring and structural design by Civitest:

https://www.civitest.pt/en/services/

Thermo-mechanical model that considers the FRC maturation, creep, shrinkage and cracking





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3.4 – Design challenges and opportunities

- Web-based information platform that includes a database ecosystem with artificial intelligence (AI) techniques for collecting the results of BSC and reliable experimental results of the scientific community in a continuous and dynamic feeding;
- Parametric studies and sensitivity analysis should be carried out for assessing the influence of the models' parameters on their predictive performance;
- Reliable methodologies for FEM-based design of FRC structures should be developed by modelling explicitly the fibres and the cracks and using an holistic vision of their structural behaviour and AI techniques

Database with information from assessing FOD, such is the case of computed tomography scanning (CTS):







The Embedded Strong Discontinuities (ESD) approach, a class of DCM, with explicit representation of fibers and cracks have a very high potential for reliable simulations, as is being demonstrated:





CONVENTION

- Chacterise the tensile behaviour of FRC (σ -w relationwship)
- Determine the necessary fracture parameters to simulate, analyse and design FRC structures



Develop a practical, efficient and automatised tool capable of determining these properties in a optimised way



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FEM model

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- Determine the necessary fracture parameters to simulate, analyse and design FRC structures



Check response

Develop a practical, efficient and automatised tool capable of determining these properties in a optimised way



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Include/exclude fct and/or Ec from the IA procedure

Possible implementation of additional models due to modular architecture

fx Numerical response based on **analytical models** reducing computational time



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Include/exclude f_{ct} and/or E_c from the IA procedure

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fx Numerical response based on **analytical models** reducing computational time



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Custom discretisation of the experimental curve











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COFIT: Theoretical aspects – underlying algorithms





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COFIT: Performance assessment



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COFIT: Performance assessment



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4 - CONCLUSIONS

Challenges and opportunities :

- AI techniques on the FRC mix design that consider the production characteristics and the structural, durability, functional and LCA requirements;
- Advanced data base for assisting on the design and use of local resources, promoting circular economy.
- Pre-fabrication based on robotics for producing with high quality control and according to advanced design tools in order to minimize natural resources, time, costs and accidents;
- Modular construction with assembling, disassembling and reuse potentialities;
- Safe connections systems for efficient assembling and disassembling of constructive elements of modular constructions;
- Building Information Modeling (BIM) Integration;
- Reliable design tools that consider the behavior of FRC from casting to SLS and ULS conditions;
- Incorporation of self-powered sensors during the prefabrication process for increasing the functional performance of the built environment, maintenance plans, and to provide well-timed information to minimize the devastating effects of extreme events;



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- > The collaboration of the fib WG2.4.1 in the BSC.



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