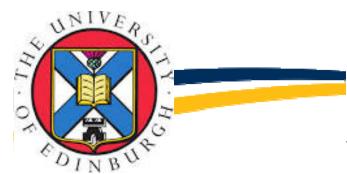
Rational design for FRP strengthened reinforced concrete structures in fire

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Rational Approaches for Fire Resistance Design of Concrete Structures
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Outline



- FRP and High Temperature
- Description of current ACI 440F recommendations
- Description of proposed changes to ACI 440F
- Loading during fire event
- Thermal modelling
- Structural modelling
- Examples

Examples of FRP



Unidirectional glass FRP bar

Carbon FRP prestressing tendon



Aramid FRP grid

Glass fibre roving

Carbon fibre roving

High strength fibres in a polymer matrix

FRPs & Fire: Primary Concerns



- Fire is recognized as a critical research need for FRP:
 - a primary factor preventing widespread application of FRPs in buildings
- Potential concerns during fire:
 - 1. Loss of strength and stiffness
 - 2. Loss of interaction (bond) w/ concrete
 - 3. Smoke generation and flame spread

Current 440F Repair Guidelines - Fire



- "The strength of externally bonded FRP systems is assumed to be lost completely... unless it can be demonstrated that the FRP temperature remains below its critical temperature (for example, FRP with a fire-protection system)"
- "The critical temperature of an FRP strengthening system should be taken as the lowest glass-transition temperature T_g of the components of the repair system,"
- "The structural member without the FRP system should possess sufficient strength to resist all applicable service loads during a fire" (1.0DL + 1.0LL)
- "The fire endurance of FRP-strengthened concrete members may be improved through the use of certain resins, coatings, insulation systems, or other methods of fire protection"
- Reference is made to ACI 216R guidelines

Proposed 440F Repair Guidelines - Fire



- "The strength of externally bonded FRP systems is assumed to be lost completely in a fire, unless it can be demonstrated that the FRP will remain effective for the required duration of the fire (for example, FRP with a fire-protection system)."
- "In most cases, the structural member without the FRP system should possess sufficient strength to resist applicable service loads during a fire" (1.2DL + 0.5LL + 0.2SL + 1.0A_k)
- "The resistance should be computed for the time period required by the member's fire-resistance rating—for example, a 2-hour fire rating—and should not account for the contribution of the FRP system unless the continued effectiveness of the FRP can be proven through testing."
- "Until better information on the properties of FRP at high temperature is available, the critical temperature can be taken as the lowest T_g of the components of the system."

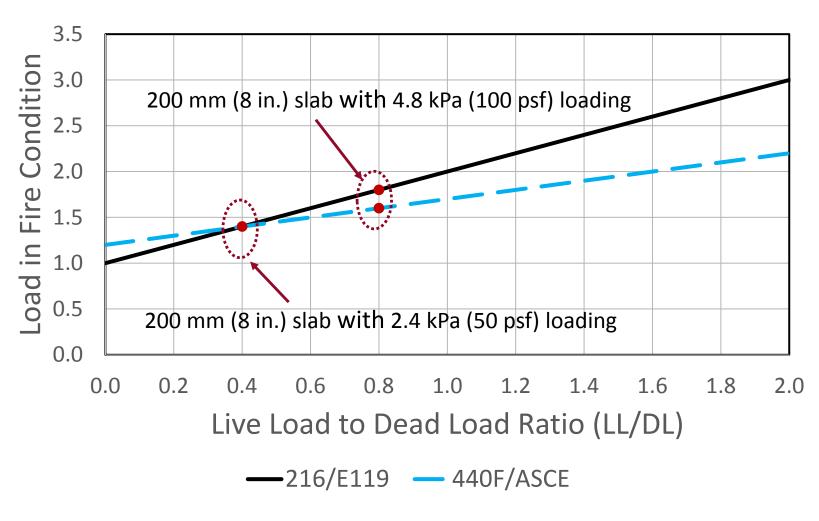
Rationale for new load factors



- Old load factors for fire (1.0DL + 1.0LL) are based on ASTM E119 provisions for fire testing (no statistical basis)
- New ASCE 7 load factors are based on probabilistic studies and are applicable for new structures (Ellingwood)¹
 - New loads have less than 5% probability of being exceeded in a given year
 - Average live load is 0.6 kPa or 12 psf (approx. 0.25 LL)

Comparison of Loading Combinations





Procedure for finding fire endurance



Necessary analysis steps:

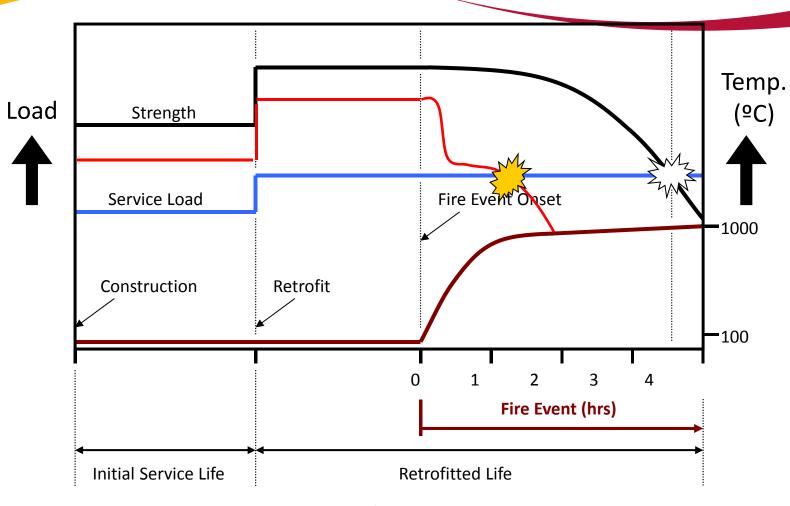
- 1. Find temperatures in member at different times during the fire exposure
 - Guides (ACI, Eurocodes), test data, analytical models, numerical tools
- 2. Evaluate reductions in strength and modulus of the materials (steel, concrete, FRP) depending on temperature in most cases the FRP will not contribute
 - ACI 216, structural Eurocodes, research literature, testing

3. Calculate member strength

- Classical plane sections analysis
- Simplified methods (e.g. Eurocode 500°C Isotherm Method)
- Finite element methods

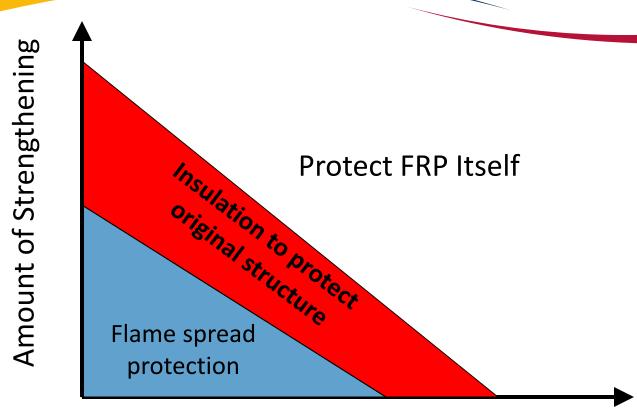
Philosophy for Fire-Safety





Time

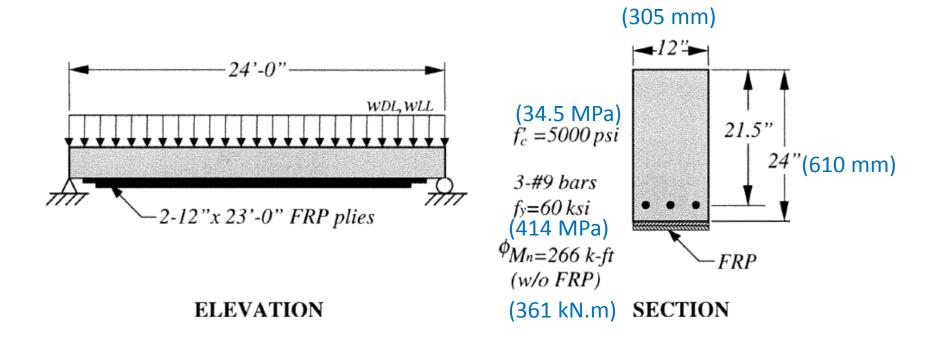




Fire resistance

Design example (after ACI 440.2R)





 $A_s = 3.0 \text{ in}^2 (1935 \text{ mm}^2)$

Analysis Approach and Assumptions



1. Find temperatures at key

- FRP determined from furnace ga
- Steel reinforcement determined
- Concrete compressive stress block

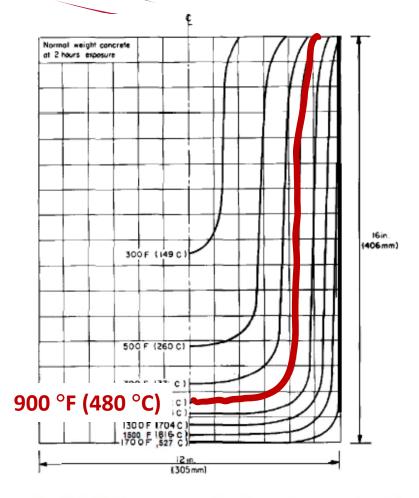


Fig. 7.2.10-Temperature distribution in normal weight concrete rectangular unit at 2 hr of fire exposure

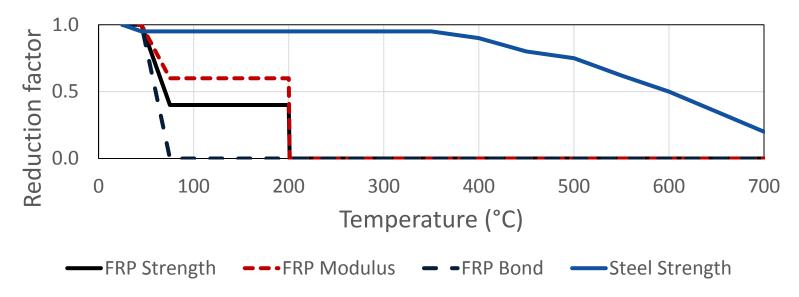
Analysis Approach and Assumptions



1. Find temperatures at key locations

- FRP determined from furnace gas temperature
- Steel reinforcement determined from ACI 216
- Concrete compressive stress block determined from ACI 216

2. Evaluate reductions in material strengths



Analysis Approach and Assumptions



1. Find temperatures at key locations

- FRP determined from furnace gas temperature
- Steel reinforcement determined from ACI 216
- Concrete compressive stress block determined from ACI 216

2. Evaluate reductions in material strengths

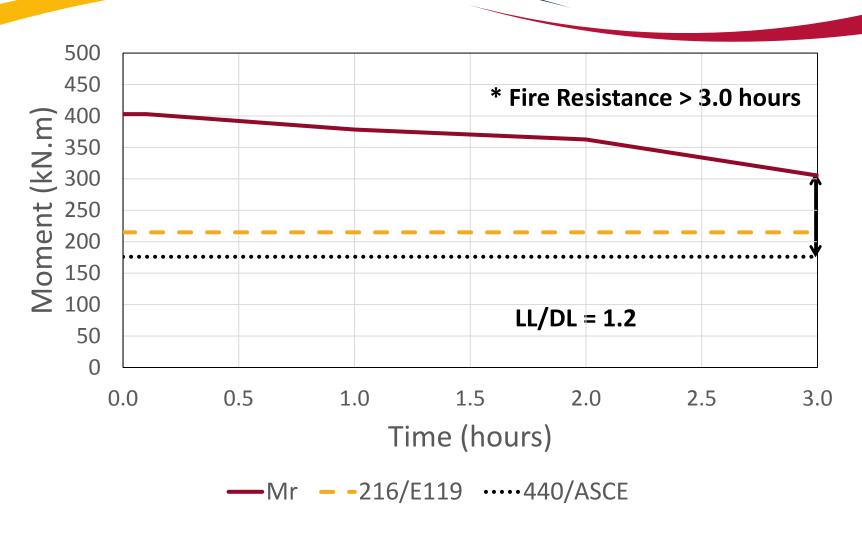
- FRP determined from Green et al. (2014) Bisby et al. (2005)
- Steel Reinforcement determined from ACI 216
- Concrete compressive stress block determined from Eurocodes

3. Calculate member strength

- Classical plane sections analysis assuming full strength in 'residual' section
- Eurocode '500°C Isotherm Method' used to treat compressive stress block width

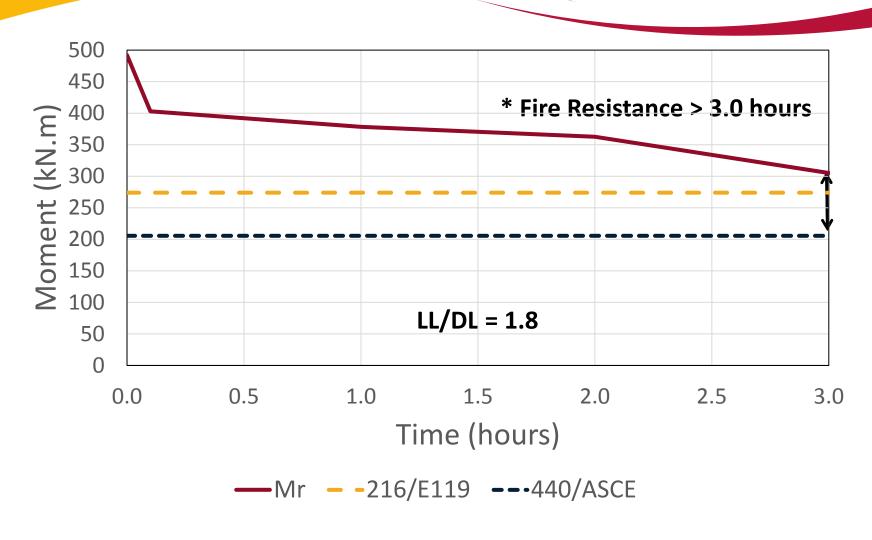
Unstrengthened beam in fire





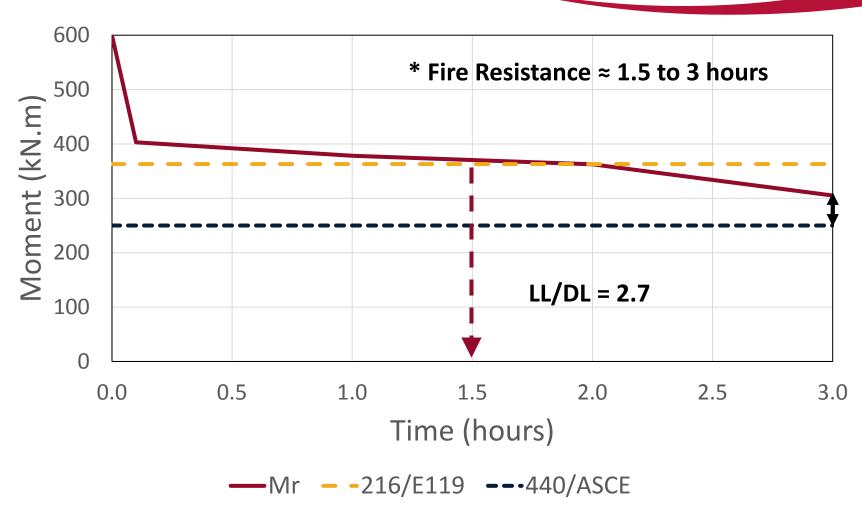
FRP Strengthened beam in fire





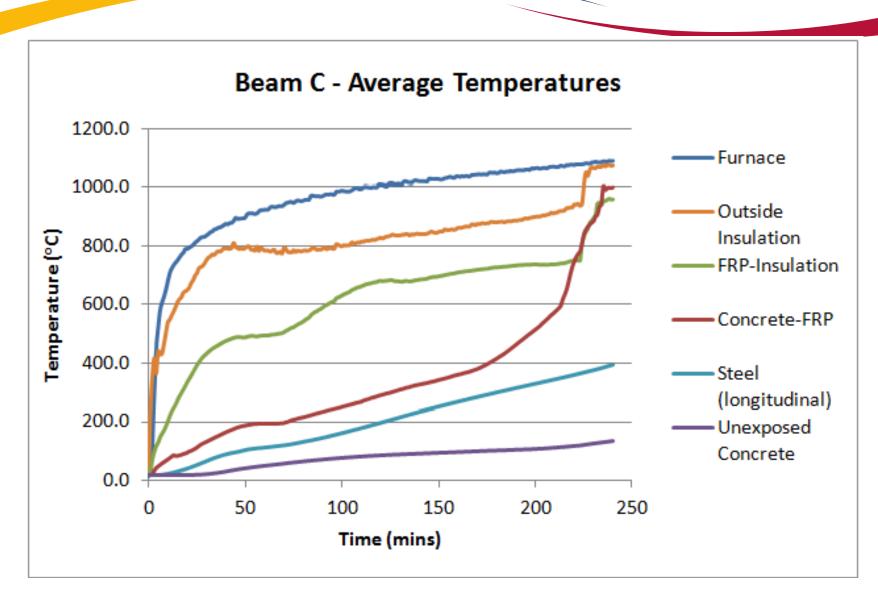
Beam FRP strengthened by 50% in fire





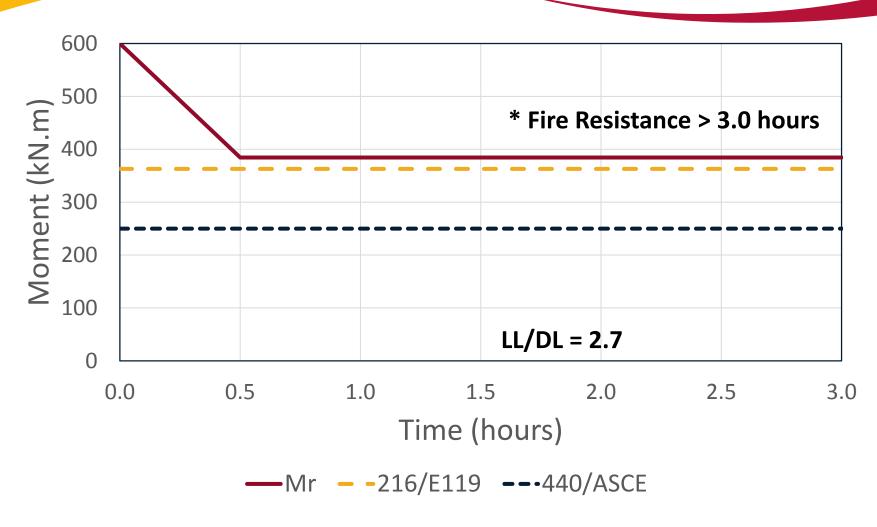
Insulation - Fire Test: Beam C (15 mm)





Beam strengthened by 50% in fire – with insulation





Conclusions



• ACI 440.2R is proposing to modify the load demand equation for the fire limit state to harmonize with ACI 562 and ASCE 7

- Practical guidance has been given on how to determine the fire resistance of FRP strengthened beams, with or without supplemental fire insulation
- In most cases it is possible for FRP strengthened concrete structural elements to achieve satisfactory fire resistances

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The Leverhulme Trust



Questions?

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