


American Concrete Institute®
Advancing concrete knowledge

The Art of Designing Ductile Concrete in the Past 50 Years: The Impact of the PCA Book and Mete A. Sozen, Part 2

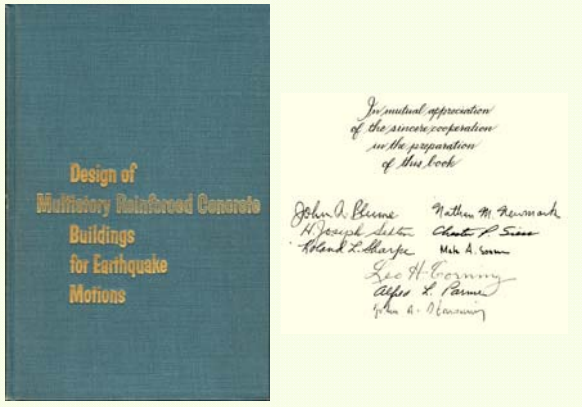
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ACI WEB SESSIONS



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ACI WEB SESSIONS



In mutual appreciation of the sincere cooperation in the preparation of this book

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Creating an option for concrete buildings

Moment-resisting frames

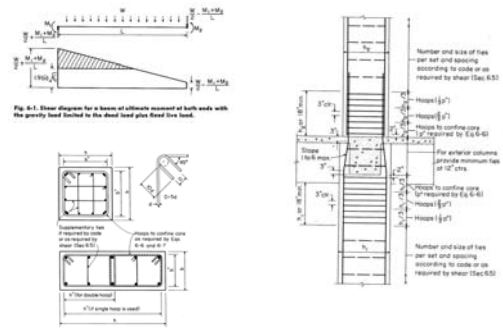
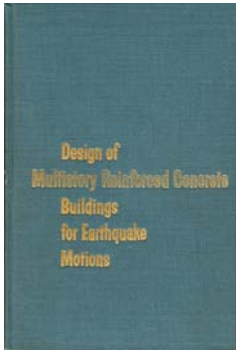


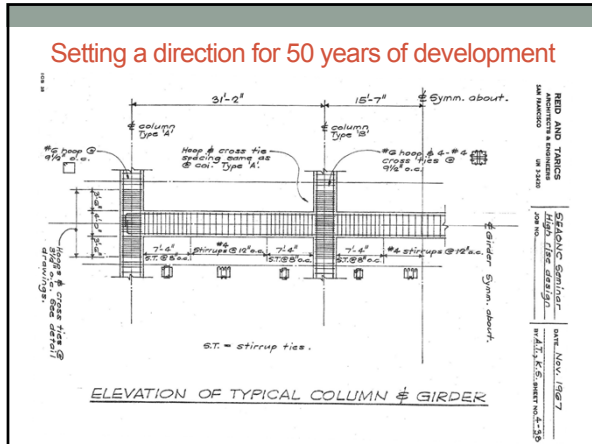
Fig. 6-1. Shear diagram for a beam of ultimate moment of both ends with the gravity load limited to the dead load plus dead live load.

Setting a direction for 50 years of development



Setting a direction for 50 years of development

- 1963 SEAC Blue Book
 - For buildings over 160 ft tall and using $K=0.67$, "...the frame shall be made of a ductile material or a ductile combination of materials such as a steel frame with moment-resistant connections or by other systems proven by tests and studies to provide equivalent energy absorption."
- 1965 SEAC Blue Book
 - **Ductile moment-resisting space frame**
 - concrete and reinforcement materials restrictions
 - beam reinforcement ratios, splice locations, anchorage provisions, capacity design for shear, web reinforcement
 - column laps at midheight, special transverse reinforcement
 - joints
 - design specified
 - column/beam moment strength ratio established
 - Walls
 - confined boundary elements



Columns

PCA

$A_{hoop} \geq \frac{h''s}{2} \rho_{spiral}$

Equivalent to ...

$$A_{sh} \geq 0.45s_b c \left(\frac{A_g}{A_{ch}} - 1 \right) \frac{f'_c}{f_{yt}}$$

ACI 318-77 through 318-11

Consecutive cross-ties engaging the same longitudinal bar have their 90-degree hooks on opposite sides of column.

45° extension

$A_{sh} \geq 0.3sb_c \left(\frac{A_g}{A_{ch}} - 1 \right) \frac{f'_c}{f_{yt}}$

The dimension x_c from centerline to centerline of its legs is not to exceed 8 inches. The term A_{ch} used in equation 21-2 is taken as the largest value of A_c .

Beams

PCA

$s_{max} = d/2$
 $\leq 16d_b$
 $\leq 12 \text{ in.}$

ACI 318-83 through 318-08

$s_{max} = d/4$
 $\leq 8d_b$ long
 $\leq 24d_b$ rest
 $\leq 12 \text{ in.}$

$V_c = 0$ in plastic hinge

ACI 318 11

$s_{max} = d/4$
 $\leq 6d_b$
 $\leq 6 \text{ in.}$

$V_c = 0$ in plastic hinge

Beam-column joints

PCA

For exterior columns provide minimum ties at 12' ctrs.

ACI 318-83 through 318-11

$\sum \phi M_{nc} \geq (6/5) \sum \phi M_{Rg}$

Structural (shear) walls

PCA

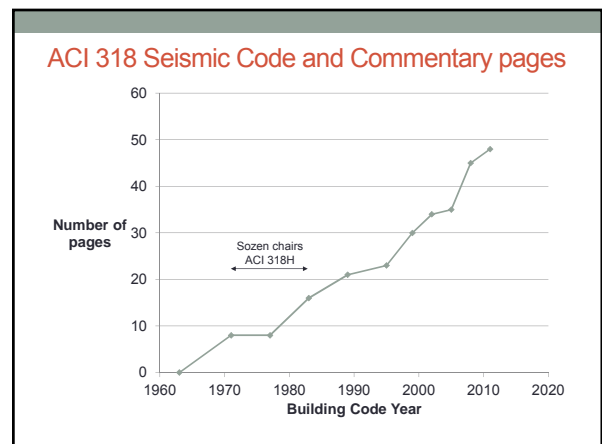
Blue Book 1965
UBC 1973
ACI 318-1983

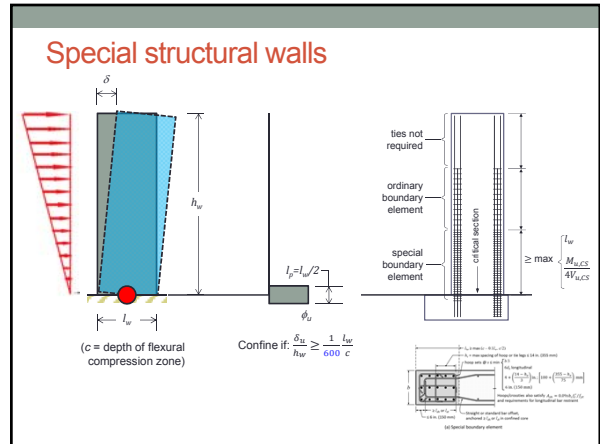
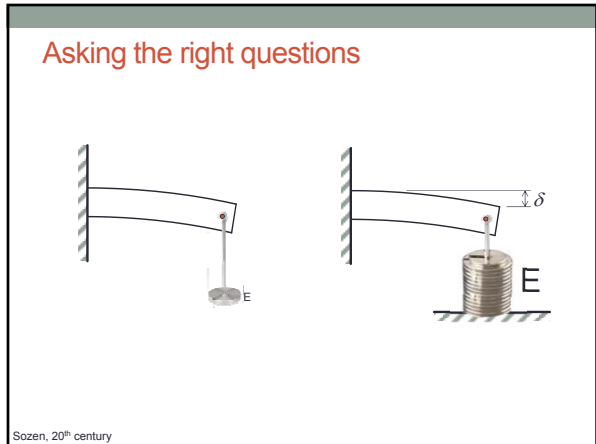
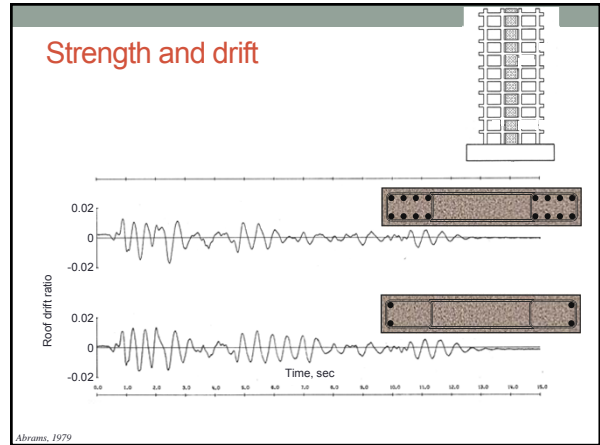
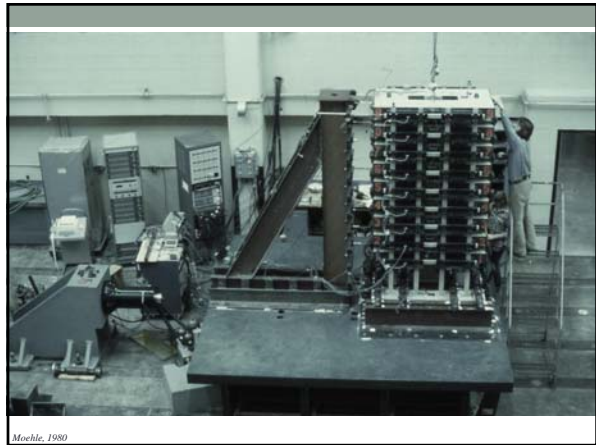
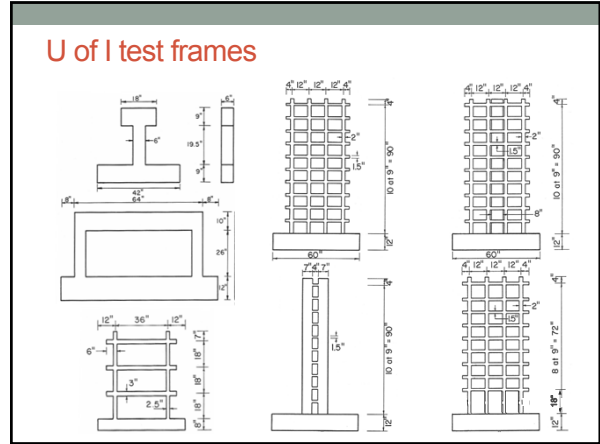
SPECIAL (Ebbesser, 1978)

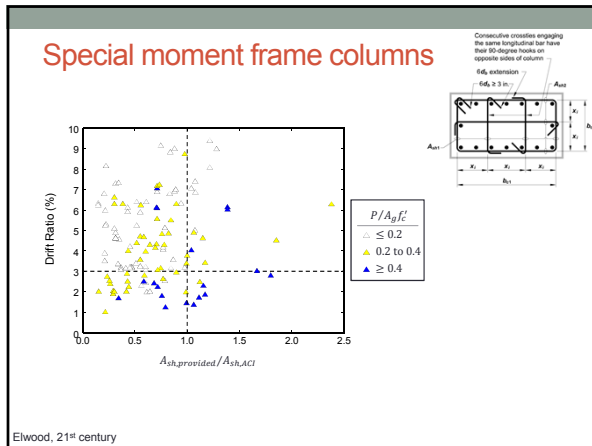
ACI 318-95 through 318-11

ties not required
ordinary boundary element
special boundary element

$\geq \max \left\{ \frac{M_{max}}{W_{pl}}, \frac{M_{max}}{W_{pl}} \right\}$







New systems/ details

PT Concrete Core

- Immediate Occupancy
 - Green Tag
 - Minimal Repair Cost and Time
- Self Centering Response
 - No Permanent Tilt
 - Tough & Damage Resistant
- Small Interstory Drift
 - Protects Façade
 - Protects Gravity Frame

Tipping|Mar

