

Report on Early-Age Cracking: Causes, Measurement, and Mitigation

Reported by ACI Committee 231

Will Hansen
Chair

Anton K. Schindler
Secretary

Akthem A. Al-Manaseer
Emmanuel K. Attiogbe
Dale P. Bentz
Joseph J. Biernacki
Matthew D. D'Ambrosia

Marwan A. Daye
Noel J. Gardner
Zachary C. Grasley
Allen J. Hulshizer
Elin A. Jensen

Mohamed Lachemi
Benjamin J. Mohr
Kamran M. Nemati
Jan Olek
Farro F. Radjy

Jussara Tanesi
Carlos C. Videla
Thomas Voight
W. Jason Weiss
Wayne M. Wilson

Early-age cracking is a challenge for the concrete industry. Materials selection, environmental conditions, and field practices all have considerable influence on the propensity for early-age cracking to occur. This document focuses on thermal- and moisture-related deformations; both are materials-related and contribute to early-age cracking. The document provides detailed reviews on the causes of deformation and cracking, test methods for assessing shrinkage and thermal deformation properties, and mitigation strategies for reducing early-age cracking.

Keywords: autogenous shrinkage; cracking; early-age; heat of hydration; measurement; microstructure; mitigation methods; shrinkage; shrinkage cracking; sustainability; thermal cracking; thermal properties.

ACI Committee Reports, Guides, Manuals, and Commentaries are intended for guidance in planning, designing, executing, and inspecting construction. This document is intended for the use of individuals who are competent to evaluate the significance and limitations of its content and recommendations and who will accept responsibility for the application of the material it contains. The American Concrete Institute disclaims any and all responsibility for the stated principles. The Institute shall not be liable for any loss or damage arising therefrom.

Reference to this document shall not be made in contract documents. If items found in this document are desired by the Architect/Engineer to be a part of the contract documents, they shall be restated in mandatory language for incorporation by the Architect/Engineer.

CONTENTS

Chapter 1—Introduction and scope, p. 231R-2

- 1.1—Introduction
- 1.2—Scope

Chapter 2—Notation and definitions, p. 231R-2

- 2.1—Notation
- 2.2—Definitions

Chapter 3—Causes of early-age deformation and cracking, p. 231R-3

- 3.1—Thermal deformation
- 3.2—Autogenous shrinkage
- 3.3—Drying shrinkage
- 3.4—Creep and stress relaxation from deformation restraint
- 3.5—Mitigation of shrinkage

Chapter 4—Test methods and assessment, p. 231R-10

- 4.1—Introduction
- 4.2—Shrinkage measurements
- 4.3—Ring test

ACI 231R-10 was adopted and published January 2010.
Copyright © 2010, American Concrete Institute.

All rights reserved including rights of reproduction and use in any form or by any means, including the making of copies by any photo process, or by electronic or mechanical device, printed, written, or oral, or recording for sound or visual reproduction or for use in any knowledge or retrieval system or device, unless permission in writing is obtained from the copyright proprietors.

- 4.4—Rigid cracking frames
- 4.5—Coefficient of thermal expansion (α_T) measurement
- 4.6—Analysis tools assessing stresses and cracking

Chapter 5—Shrinkage control, p. 231R-27

- 5.1—Introduction
- 5.2—Expansive additives
- 5.3—Shrinkage-reducing admixtures
- 5.4—Internal curing

Chapter 6—References, p. 231R-39

- 6.1—Referenced standards and reports
- 6.2—Cited references

CHAPTER 1—INTRODUCTION AND SCOPE

1.1—Introduction

ACI Committee 231 defines “early age” as the period after final setting, during which properties are changing rapidly. For a typical Type I portland-cement concrete moist cured at room temperature, this period is approximately 7 days. This document, however, includes discussions of early-age effects beyond 7 days. It is important to understand how concrete properties change with time during early ages and how different properties are interrelated, which may not be the same as for mature concrete. It is also important to understand how these early-age changes influence the properties of concrete at later ages. The temperature history at early ages has a strong effect on whether concrete may develop its potential strength. Poor early-age curing has been demonstrated to detrimentally affect the strength, serviceability, and durability.

Concrete structures change volume due to the thermal- and moisture-related changes. This may be detrimental because substantial stresses may develop when the concrete is restrained from moving freely. This is particularly important at early ages while the concrete has a low tensile strength. Therefore, the assessment and control of early-age cracking should be based on several factors, such as age-dependent material properties, thermal- and moisture-related stresses and strains, material viscoelastic behavior, restraints, and environmental exposure.

Temperature control in concrete during the early stages of hydration is essential for achieving early strength as well as ultimate strength and to eliminate or minimize uncontrolled cracking due to excessive mean peak temperature rise and thermal gradients (ACI 207.1R and 207.2R). Of particular importance in determining the risk of early-age cracking of any concrete member is an assessment of the magnitude of the stresses generated in the concrete as a result of restraint to thermally induced movement. In general, there are two types of restraint: external and internal. External restraints are caused by support conditions, contact with adjacent sections, applied load, reinforcement, and base friction in the case of concrete slabs-on-ground. Internal restraint is a manifestation of the residual stresses that develop as a result of nonlinear thermal and moisture gradients within a cross section.

New methods were developed and older methods rediscovered for evaluating stress and strain development and assessing

cracking risk in concrete mixtures under realistic exposure conditions. Categories of evaluation methods discussed in this document include restrained and unrestrained volume change tests, coefficient of thermal expansion tests, and tools for assessing stress development and cracking potential. Some of these evaluation methods have been standardized.

Mitigation methods have focused mainly on reducing the autogenous (moisture-related) component of the early-age stresses or compensating for the early-age shrinkage by employing expansive cement. In the former case, both shrinkage-reducing admixtures (SRAs) and internal curing have been demonstrated to reduce the magnitude of the early-age shrinkage of specimens cured under sealed, isothermal conditions.

The prevention or mitigation of early-age cracking will improve the long-term durability of concrete structures and, therefore, enhance their sustainability by increasing the service life.

1.2—Scope

This document reviews the causes of early-age deformation and cracking. The test methods for quantifying the early-age stress development and hence the risk of cracking due to thermal and moisture conditions are described. Mitigation methods for stress reduction are also discussed.

CHAPTER 2—NOTATION AND DEFINITIONS

2.1—Notation

C	= cement factor (content) for concrete mixture, lb/yd ³ (kg/m ³)
C_f	= correction factor accounting for the change in length of the measurement apparatus with temperature, $0.56 \times 10^{-6}/^{\circ}\text{F}$ ($1 \times 10^{-6}/^{\circ}\text{C}$)
CS	= chemical shrinkage of cement (mass of water/mass of cement)
D	= moisture diffusion coefficient of concrete
dT/dt	= temperature change
$d\varepsilon_{hygral}/dt$	= rate of nonthermal deformation due to autogenous shrinkage, drying shrinkage, or both
$E(t)$	= Young’s modulus at time t , psi (MPa)
E_c	= creep-adjusted modulus of elasticity of concrete, psi (MPa)
$E_c(t)$	= age-dependent elastic modulus of concrete
E_{CON}	= elastic modulus of concrete
E_{steel}	= modulus of elasticity of steel ring, psi (MPa)
$erfc$	= complementary error function
f	= geometry function (Moon and Weiss 2006)
h	= humidity (0 to 1)
G	= coefficient relating stress to steel ring strain 10.44×10^6 psi (72.2 GPa) for the ASTM ring)
\hat{K}	= stress amplification factor
K_r	= degree of restraint factor
L	= length
L_o	= measured length of specimen at room temperature, in. (mm)
M_{LWA}	= mass of (dry) fine lightweight aggregate needed per unit volume of concrete, lb/yd ³ (kg/m ³)
R	= degree of restraint